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Theoretical Analysis on Deflection and Bearing Capacity of Prestressed Bamboo-Steel Composite Beams

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ABSTRACT

A theoretical analysis of upward deflection and midspan deflection of prestressed bamboo-steel composite beams is presented in this study. The deflection analysis considers the influences of interface slippage and shear deformation. Furthermore, the calculation model for flexural capacity is proposed considering the two stages of loading. The theoretical results are verified with 8 specimens considering different prestressed load levels, load schemes, and prestress schemes. The results indicate that the proposed theoretical analysis provides a feasible prediction of the deflection and bearing capacity of bamboo-steel composite beams. For deflection analysis, the method considering the slippage and shear deformation provides better accuracy. The theoretical method for bearing capacity matches well with the test results, and the relative errors in the serviceability limit state and ultimate limit state are 4.95% and 5.85%, respectively, which meet the accuracy requirements of the engineered application.

KEYWORDS

Bamboo scrimber; composite beam; prestress; deflection; bearing capacity

1 Introduction

Bamboo is the most important plant resource in the world, characterized by a fast-growing rate, renewability, biodegradability, and carbon sequestration ability [1]. Bamboo scrimber is a new sustainable and renewable bamboo-based construction product, which is a breakthrough achieved by the bamboo processing industry [2,3]. Therefore, numerous bamboo materials [4,5], bamboo-based members [6–8], and structures [9,10] are developed and investigated.

Nevertheless, the flexural members made by bamboo scrimber had higher deflection compared with concrete and steel members [11–13]. In this case, the bamboo structure members strengthened by other materials such as steel reinforcement [14,15], steel plates [16,17], and FRP grids [18–20] were designed and investigated in recent years.

On the other hand, an I-shaped section is a type of highly efficient beam section, which can economically enhance the flexural stiffness and bearing capacity compared with a solid rectangular section [21]. Chen et al.



[22] proposed composite I-beams made of timber and bamboo; the bearing capacity and stiffness of these beams increased by 44.8% and 23.4% on average compared with ordinary timber beams. Ghanbari-Ghazijahani et al. [23] investigated timber I-beams with different strengthening strategies, and the test results showed that the load-carrying capacity of strengthened beams could be increased by 70%.

Thin-walled cold-formed steel is widely used in modern structures with high strength and stiffness, while the steel members tend to buckle before achieving ultimate strength [24]. In this case, a new bamboo-steel composite beam is proposed [25,26]. The composite beams consist of bamboo scrimber plates and thin-walled cold-formed steel channels, which combine the best features of the two materials while avoiding the disadvantages.

Large-span structure is an important form of the modern structural system [27]. To expand the application scope of the bamboo-steel composite structure and make full use of the high strength of bamboo and steel, a novel prestressed bamboo-steel composite beam is proposed, which is composed of thin-walled steel channel and bamboo scrimber panels and strengthened by externally prestressed strands (Fig. 1). The deflection and bearing capacity of large-span beams are important design parameters for the application. Numerical simulation and theoretical analysis on the flexural rigidity and bearing capacity of the prestressed reinforced bamboo/timber beams have been conducted in many studies [28–30]. Tian et al. [31] proposed theoretical equations for predicting the flexural resistance of laminated bamboo lumber beams strengthened with prestressed embedded steel bars based on the observed strain and stress distributions. Lago et al. [32] conducted a semi-analytical iterative procedure for the solution of the geometrical nonlinear problem of unbonded post-tensioning to capture and predict the experimental behavior. These papers focus on the studies of prestressed bamboo/timber beams with numerical simulation and complex theoretical models, which provide good references for our study. In this study, a theoretical analysis of deflection and bearing capacity is proposed in this paper, and verification with 8 specimens considering different prestressed load levels, load schemes, and prestress schemes. The paper can offer a reference for the application of prestressed bamboo-steel composite beams.

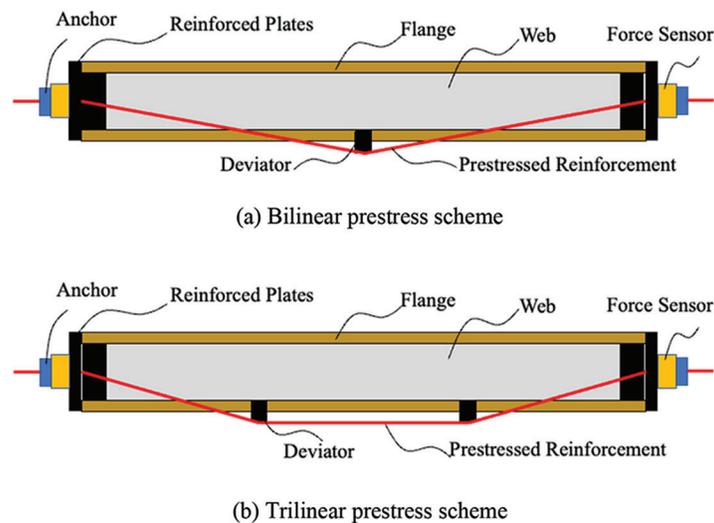


Figure 1: Diagram of prestressed bamboo-steel composite beam

2 Test Overview

2.1 Test Specimens

To analyze the bearing capacity and deflection of prestressed bamboo-steel composite beams, 8 I-shaped composite beams were prepared for the test. The specimens are shown in Fig. 2a. The specimens have the same section dimension (Fig. 2b). The length of specimens was 3800 mm, while the calculated span was 3500 mm. The main design parameters were prestressed load levels, prestress schemes, and load schemes. The detailed parameters of specimens are listed in Table 1.

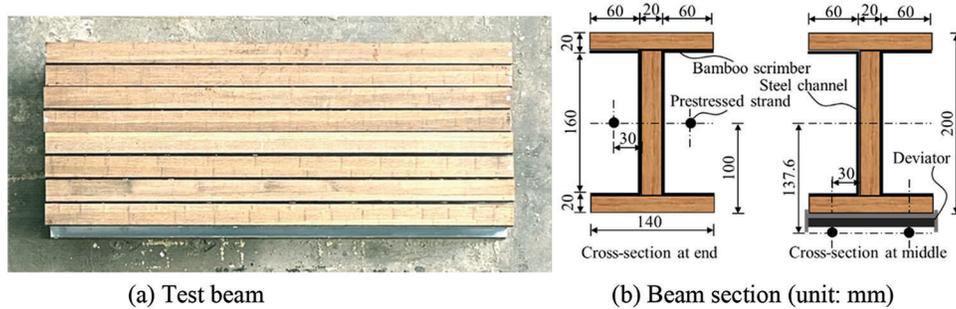


Figure 2: Specimens of composite beams

Table 1: Details of specimen parameters

Specimens	Prestressed load (kN)	Prestress scheme	Load scheme
B-20-I-1	20	I	1
B-30-I-1	30	I	1
B-20-I-2	20	I	2
B-30-I-2	30	I	2
B-20-II-1	20	II	1
B-30-II-1	30	II	1
B-20-II-2	20	II	2
B-30-II-2	30	II	2

Note: I and II represent bilinear and trilinear prestress schemes, 1 and 2 represent three-point and four-point bending load schemes.

2.2 Material Properties

The bamboo-steel composite beam is made of cold-formed thin-walled steel and bamboo scrimber bonded by adhesive. The adhesive is Henkel EA3162A/B component epoxy adhesive, manufactured by LOCTITE. The prestressed strands are made of seven-strand wire, whose nominal diameter is 15.2 mm. The main mechanical properties of the three materials are shown in Table 2, in which the strength and elastic modulus of thin-walled steel and bamboo scrimber are measured according to standards [33] and [34], respectively. The mechanical properties of prestressed strands are provided by the manufacturer.

2.3 Specimen Preparation

The fabrication of the specimens is as follows: first, the surfaces of the bamboo panel and steel channel were polished with polishers and cleaned with alcohol wipes (Fig. 3a). Then, the web surfaces of the steel channel and bamboo panel were evenly coated with adhesive. The steel channel and bamboo web were

bonded together to manufacture the web skeleton of the composite beams. Ballasts and fixtures were employed to fix the web skeleton for seven days to ensure the quality of the interface between the bamboo and steel channel (Fig. 3b). Third, the bamboo flange and steel flange were bonded with the same treatments above (Fig. 3c). The finished specimens without prestressed strands are shown in Fig. 2a.

Table 2: Mechanical properties of materials

Material	Strength (MPa)	Elastic modulus (MPa)
Bamboo scrimber	93.47 (Compression) 124.04 (Tension)	15673
Thin-walled steel	284 (Yield tension) 378 (Ultimate tension)	2.0×10^5
Prestressed strands	1860 (Ultimate tension)	1.95×10^5



Figure 3: Specimens preparation

2.4 Prestress Scheme

As shown in Fig. 4, two external prestressed strands are employed to apply prestress to bamboo-steel composite beams. The prestressing scheme can be divided into a bilinear prestress scheme and a trilinear prestress scheme according to different layouts of prestressed strands. The deviators are adopted to change the layouts of strands. The deviators were installed in the middle of the beams for the bilinear prestress scheme. The distance between the deviator and the adjacent support point is 1200 mm for the trilinear prestress scheme.

2.5 Load Scheme

A universal reaction frame was adopted for the test, and the vertical load was applied through a hydraulic jack and measured by a load sensor. The load control strategy with the step of 5 kN was employed for loading. There are two schemes for vertical loading according to Fig. 4. For the three-point bending load scheme, and the vertical load is directly applied to the upper flange of the midspan section. While for the four-point bending load scheme, a rigid steel beam is used to achieve symmetric two-point loading, and the distance from the loading point to the adjacent support point is 1200 mm.

3 Deformation of Prestressed Bamboo-Steel Composite Beam

3.1 Additional Deflection Caused by Interface Slippage

Previous studies about the flexural behavior of bamboo-steel composite beams have indicated that slight slippage occurred between bamboo and steel [35]. The slippage has a certain influence on the deformation of the composite beams. The additional deflection caused by slippage should not be ignored [36].

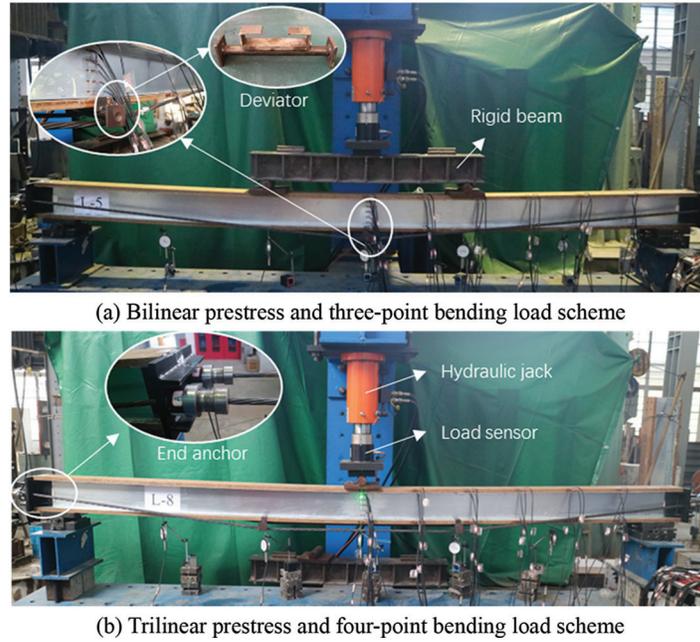


Figure 4: Prestress and load scheme

The calculation method of additional deflection has been proposed and matches well with the test results according to the reference [37]. The formula can be expressed as follows:

$$\Delta f_{s1} = \frac{\beta(\alpha L - 2)}{\alpha h} F \quad (1.1)$$

$$\Delta f_{s2} = \frac{\beta(\alpha L - 2\alpha b - 2e^{-\alpha b})}{\alpha h} F \quad (1.2)$$

$$\alpha^2 = K \left(\frac{1}{E_b A_b} + \frac{Z^2}{EI} \right) \quad (1.3)$$

$$\beta = \frac{Z}{K \left(\frac{EI}{E_b A_b} + Z^2 \right)} \quad (1.4)$$

$$Z = (h_f + h_w)/2 \quad (1.5)$$

$$EI = E_b I_b + E_s I_s \quad (1.6)$$

where Δf_{s1} and Δf_{s2} are the additional deflections caused by one-point concentrated load and two-point symmetric load, respectively. F is the total load applied to the beam, L is the calculated span of beams, b is the distance between the two symmetric load points, h is the height of the beam section, K is the slip stiffness of interface among the unit length, A_b is the sectional area of bamboo, h_f and h_w are the height of sectional flange and web, respectively. E and I are the elastic modulus and section inertia moment, respectively, and subscripts b and s represent bamboo and steel.

3.2 Additional Deflection Caused by Shear Deformation

The flexural behaviors of composite beams will be determined by the combined effects of bending moment and shear force [38,39]. The additional deflection caused by shear deformation should be considered when calculating the deformation of composite beams. According to the principle of virtual force in structural mechanics, the deflection deformation caused by shear force can be calculated and solved by the graph multiplication method, as shown in Fig. 5. The additional deflection Δf_{v1} caused by shear deformation under the concentrated load and Δf_{v2} caused by shear deformation under the two-point symmetric load can be calculated as follows:

$$\Delta f_{v1} = \int_0^L \frac{kV_i V_p}{GA} ds = \sum \frac{k w_i y_i}{GA} = \frac{k(w_1 y_1 + w_2 y_2)}{GA} = \frac{kL}{4GA} F \tag{2.1}$$

$$\Delta f_{v2} = \frac{k(w_3 y_3 + w_4 y_4)}{GA} = \frac{ka}{2GA} F \tag{2.2}$$

$$k = \frac{A}{I^2} \int \frac{s^2}{b^2} dA \tag{2.3}$$

$$GA = G_b A_b + G_s A_s \tag{2.4}$$

where k is the uniformity coefficient of shear stress distribution, which is only related to the shape of the section, and the specific value can be obtained from Eq. (2.3). G and A are the shear modulus and section area, respectively, and a is the distance between the load point and the adjacent support point.

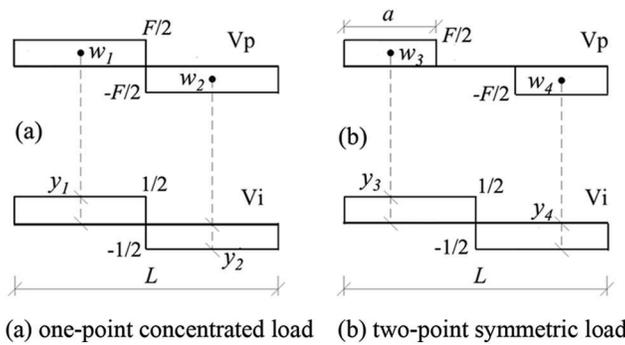


Figure 5: Calculation diagram of graph multiplication method

3.3 Upward Deflection Caused by Prestress

The application of prestressed reinforcement may cause an upward deflection for the beam. According to the equivalent theory of statics, the effect of prestressed reinforcement can be equivalent to upward forces acting on the deviators. Therefore, the bilinear prestress scheme can be seen as a concentrated upward force acting on the midspan of the beam, and the trilinear prestress scheme can be regarded as two symmetric upward forces acting upon the deviators (Fig. 6).

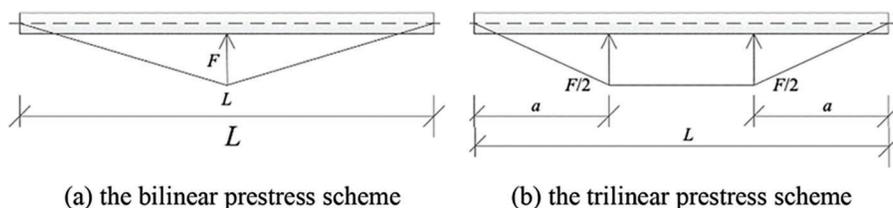


Figure 6: Equivalent load for prestressed reinforcement

Without considering the influence of additional deflection, the mid-span deflection of composite beams can be expressed according to the mechanics of materials as follows:

$$y_1 = \frac{L^3}{48EI} F \quad (3.1)$$

$$y_2 = \frac{a(3L^2 - 4a^2)}{48EI} F \quad (3.2)$$

where y_1 is the midspan deflection of the beam under the bilinear prestress scheme, y_2 is the one under the trilinear prestress scheme.

Therefore, the midspan upward deflection r can be expressed as Eq. (4). The total midspan upward deflection R considering the slippage and shear deformation can be expressed as Eq. (5).

$$r_i = y_i \quad (4)$$

$$R_i = r_i + \Delta f_{si} + \Delta f_{vi} \quad (5)$$

where $i = 1$ or 2 , represents the bilinear prestress scheme or the trilinear prestress scheme, respectively.

3.4 Deflection

According to the test overview above, the midspan deflection of the prestressed composite beam under the four conditions (i.e., the bilinear prestress and three-point bending scheme, the bilinear prestress and four-point bending load scheme, the trilinear prestress and three-point bending load scheme, the trilinear prestress and four-point bending load scheme) are discussed in this paper.

3.4.1 Bilinear Prestress Scheme

The deformation of composite beams under the external load will cause the stress increment of the prestressed reinforcements, in turn, the stress increment may offset the deformation of beams. Therefore, the final deflection of the prestressed beam is the interaction caused by external load and stress increment of prestressed reinforcements.

The load condition and deformation of beams under the bilinear prestress and three-point bending load scheme are shown in Fig. 7a. The midspan deflection x under the external load causes the stress increment Q of a single strand, and the stress increment causes the upward load increment ΔN , the specific expressions are as follows:

$$Q = \varepsilon E_p A_p \quad (6.1)$$

$$\varepsilon = \frac{\Delta l}{l_0} = \frac{l_1 - l_0}{l_0} \quad (6.2)$$

$$l_0 = 2\sqrt{(L'/2)^2 + d^2} \quad (6.3)$$

$$l_1 = 2\sqrt{(L'/2)^2 + (d + x)^2} \quad (6.4)$$

$$\Delta N = 2 \times 2Q \sin \alpha \quad (7.1)$$

$$\sin \alpha = \frac{d}{\sqrt{(L'/2)^2 + d^2}} \quad (7.2)$$

where l_0 and l_1 are the original length and final elongation length of prestressed reinforcement, respectively. L' is the total length of the composite beam, d is the centroid distance between the beam and prestressed reinforcement, E_p and A_p are the elastic modulus and area of prestressed reinforcement, respectively.

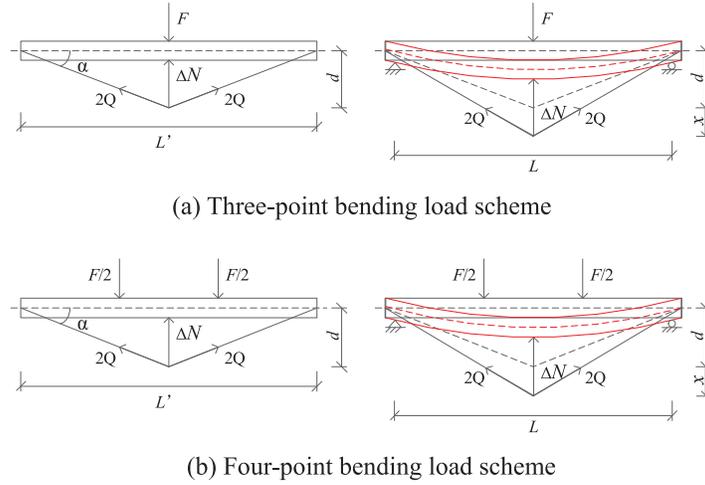


Figure 7: Deformation of bilinear prestress scheme

According to the superposition principle and Eq. (3.1), the deflection x can be expressed:

$$x_1 = \frac{L^3}{48EI} (F - \Delta N) \quad (8)$$

Similarly, the deformations of beams under the bilinear prestress and four-point bending load scheme are shown in Fig. 7b. The stress increment Q and upward load ΔN can be calculated through Eqs. (6.1) and (7.1). The deflection caused by external load and upward prestress equivalent load can be calculated by Eqs. (3.1) and (3.2), respectively. The deflection can be calculated based on the superposition principle:

$$x_2 = \frac{a(3L^2 - 4a^2)}{48EI} F - \frac{L^3}{48EI} \Delta N \quad (9)$$

The deflection can be obtained by solving the equations about x , the midspan deflection f_{1j} of prestressed beams under the bilinear prestress scheme considering slippage and shear deformation can be calculated as follows:

$$f_{1j} = x_j - R_1 + \Delta f_{sj} + \Delta f_{vj} \quad (10)$$

where $j = 1$ or 2 , represents a three-point bending load or four-point bending load scheme, respectively. ΔN is the result of combined actions, which has considered the influence of slippage and shear deformation. While in Eq. (10), the external load F should still consider the influence of slippage and shear deformation.

3.4.2 Trilinear Prestress Scheme

The load condition and deformation of beams under the trilinear prestress and three-point bending load scheme are shown in Fig. 8a. The original length l_0 and final elongation length l_1 of the prestressed reinforcement can be obtained:

$$l_0 = 2\sqrt{(a'/2)^2 + d^2} + b \quad (11.1)$$

$$l_1 = 2\sqrt{(a'/2)^2 + (d+x)^2} + b \quad (11.2)$$

where a' is the distance between the deviator and the adjacent support point, and b is the distance between the two deviators.

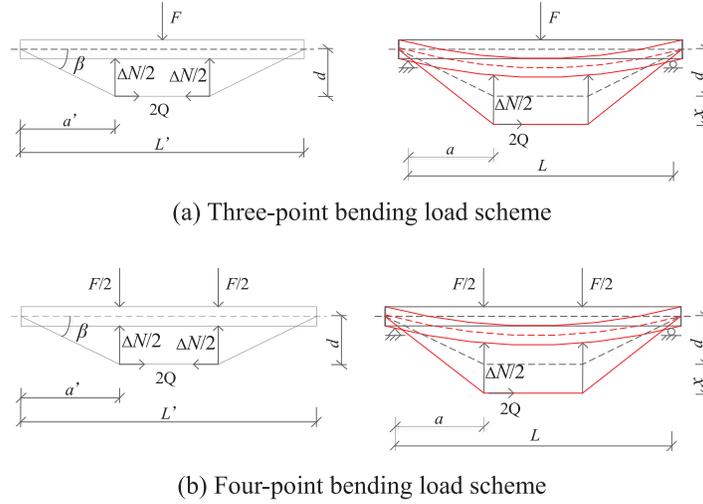


Figure 8: Deformation of trilinear prestress scheme

Substituting Eqs. (11.1) and (11.2) into Eqs. (6.1) and (7.1), the load increment ΔN caused by stress increment Q can be calculated as follows:

$$\Delta N = 4Q \sin \beta \quad (12.1)$$

$$\tan \beta = \frac{d}{a'} \quad (12.2)$$

The midspan deflection x can be obtained based on the superposition principle:

$$x_1 = \frac{L^3}{48EI} F - \frac{a(3L^2 - 4a^2)}{48EI} \Delta N \quad (13)$$

Similarly, the deformations of beams under the trilinear prestress and four-point bending load scheme are shown in Fig. 8b. The midspan deflection x can be calculated as follows:

$$x_2 = \frac{a(3L^2 - 4a^2)}{48EI} (F - \Delta N) \quad (14)$$

The deflection can be obtained by solving the equations about x , the midspan deflection f_{2j} of prestressed beams under the trilinear prestress scheme considering slippage and shear deformation can be calculated as follows:

$$f_{2j} = x_j - R_2 + \Delta f_{sj} + \Delta f_{vj} \tag{15}$$

where, $j = 1$ or 2 , represents a three-point bending load scheme or four-point bending load scheme, respectively.

4 Bearing Capacity of Prestressed Bamboo-Steel Composite Beams

4.1 Stress Increment of Prestressed Reinforcement

The prestressed bamboo-steel composite beams can be regarded as one-degree of statically indeterminate structures composed of external prestressed reinforcement and bamboo-steel composite beams. The stress increment of prestressed reinforcement can be solved by the force method in structural mechanics. To simplify the theoretical analyses, some basic assumptions are introduced:

- (1) The composite beam sections conform to the plane section assumption.
- (2) The deformation of the end anchorage device is neglected.
- (3) The prestressed reinforcement is considered as an elastic body.
- (4) The stress of the prestressed reinforcement is assumed to be equal along the length direction.

4.1.1 Bilinear Prestress Scheme

The basic diagram of the bilinear prestress and three-point bending load scheme for the force method is shown in Fig. 9a. The basic equation for the force method is expressed as follows:

$$\delta_{11} \cdot \Delta T + \Delta_{1p} = 0 \tag{16}$$

where ΔT is the stress increment of prestressed reinforcement, δ_{11} is the displacement caused by unit load, and Δ_{1p} is the displacement caused by the external load. The δ_{11} and Δ_{1p} can be calculated according to the virtual work principle.

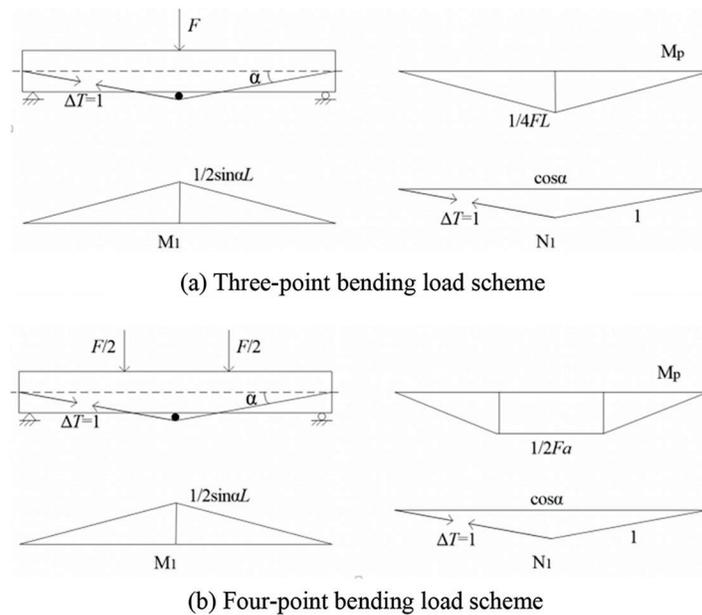


Figure 9: Basic diagram of bilinear prestress scheme

$$\delta_{11} = \int_0^L \frac{M_1^2}{EI} dx + \int_0^L \frac{N_1^2}{EA} dx + \int_0^{L_1} \frac{N_1^2}{E_p A_p} dx = \frac{\sin^2 \alpha \cdot L^3}{12EI} + \frac{\cos^2 \alpha \cdot L}{EA} + \frac{L_1}{E_p A_p} \quad (17.1)$$

$$\Delta_{1p} = \int_0^L \frac{M_1 M_P}{EI} dx = -\frac{\sin \alpha \cdot L^3}{24EI} F \quad (17.2)$$

where, $EA = E_b A_b + E_s A_s$, α can be calculated according to Eq. (7.2), L_1 is the total length of prestressed reinforcement.

The basic diagram of the bilinear prestress and four-point bending load scheme is shown in Fig. 9b. The δ_{11} can be calculated according to Eq. (17.1), while Δ_{1p} can be obtained as follows:

$$\Delta_{1p} = \int_0^L \frac{M_1 M_P}{EI} dx = -\frac{\sin \alpha \cdot a(3L^2 - 4a^2)}{24EI} F \quad (18)$$

4.1.2 Trilinear Prestress Scheme

The basic diagram of the trilinear prestress and three-point bending load scheme for the force method is shown in Fig. 10a. The δ_{11} and Δ_{1p} can be calculated as follows:

$$\delta_{11} = \frac{\tan^2 \beta \cdot a^2}{EI} \left(L - \frac{4}{3} a \right) + \frac{\cos^2 \beta \cdot L}{EA} + \frac{L_1}{E_p A_p} \quad (19.1)$$

$$\Delta_{1p} = -\frac{\tan \beta \cdot a(3L^2 - 4a^2)}{24EI} F \quad (19.2)$$

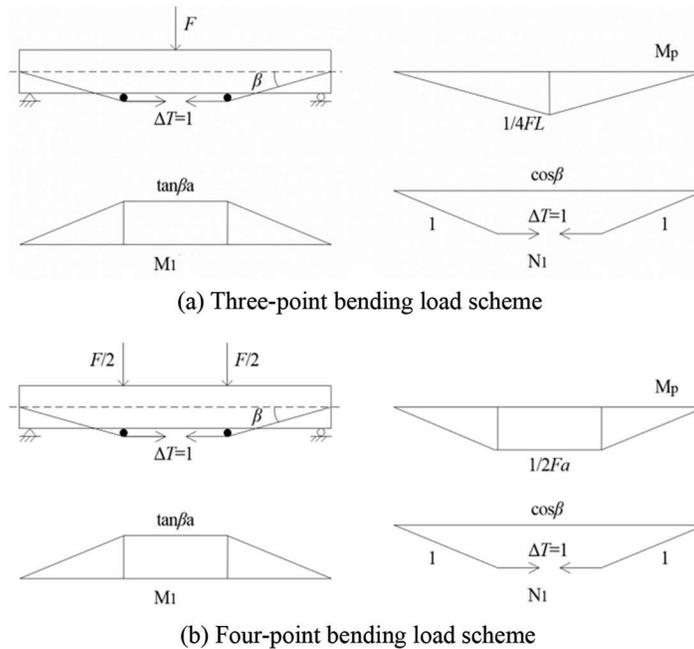


Figure 10: Basic diagram of trilinear prestress scheme

The basic diagram of the trilinear prestress and four-point bending scheme is shown in Fig. 10b. The δ_{II} can be calculated according to Eq. (19.1), while Δ_{Ip} can be obtained as follows:

$$\Delta_{Ip} = -\frac{\tan\beta \cdot a^2(3L - 4a)}{6EI}F \quad (20)$$

Thus, the stress increment ΔT under the different prestress and load schemes can be obtained by substituting the corresponding δ_{II} and Δ_{Ip} into Eq. (16).

4.2 Flexural Load-Bearing Capacity

To analyze the flexural load-bearing capacity of the prestressed composite beams, the whole loading process can be divided into two stages. In the first stage, the deflection of beams changes from a precamber state to zero. In the second stage, the deflection of beams changes from zero to the positive ultimate limit state with the combined action of external load and prestressed reinforcement. Thus, the critical state is when the deflection of composite beams reaches zero.

4.2.1 Bearing Capacity in the First Stage

In the first stage, when the bearing capacity reaches the maximum, the deflection of the composite beams is zero, i.e., $y_p + y_s = 0$, where y_p is the deflection of composite beams without prestressed reinforcement caused by the external load, y_s is the upward deflection caused by prestress.

For the bilinear prestress and four-point bending load scheme, the y_p and y_s can be expressed as follows:

$$y_p = \frac{a(3L^2 - 4a^2)}{48EI}F_1 \quad (21.1)$$

$$y_s = \frac{L^3}{48EI}\Delta N \quad (21.2)$$

where F_1 is the load bearing-capacity of the prestressed bamboo-steel composite beams in the first stage. Thus, the F_1 for the bilinear prestress and four-point bending load scheme can be obtained:

$$F_1 = \frac{L^3}{a(3L^2 - 4a^2)}\Delta N \quad (22.1)$$

Similarly, the bearing capacity for the other three schemes can be expressed:

For the bilinear prestress and three-point bending load scheme:

$$F_1 = \Delta N \quad (22.2)$$

For the trilinear prestress and three-point bending load scheme:

$$F_1 = \frac{a(3L^2 - 4a^2)}{L^3}\Delta N \quad (22.3)$$

For the trilinear prestress and four-point bending load scheme:

$$F_1 = \Delta N \quad (22.4)$$

where ΔN is the total upward load increment caused by prestressed reinforcement, which can be calculated based on Eqs. (7.1) or (12.1) according to different prestressing schemes.

4.2.2 Bearing Capacity in the Second Stage

The ultimate bending moment M_u of the composite beam in the second stage can be calculated according to the superposition principle.

$$M_u = M_s + M_b \quad (23.1)$$

$$M_s = \gamma_s f_u W_s \quad (23.2)$$

$$M_b = \gamma_b \sigma_b W_b \quad (23.3)$$

where M_s and M_b are the ultimate bending moment provided by steel channel and bamboo, respectively. W_s and W_b are the elastic section modulus of steel and bamboo, respectively. f_u is the ultimate stress of steel. Some references [25,35] indicated that the steel reached the plastic stage in the ultimate limit state, and the plastic ratio of steel γ_s ($\gamma_s = 1.05$) was introduced to consider the plastic behavior. σ_b is the stress of bamboo, which can be calculated: $\sigma_b = \varepsilon_b E_b$. According to the reference, slight debonding could be observed during the bamboo-steel composite beam test, so the strength reduction factor γ_b ($\gamma_b = 0.95$) is introduced.

The ultimate bending moment M of prestressed composite beams also can be expressed according to the external load and prestressed reinforcement.

For the bilinear prestress and three-point bending load scheme:

$$M = \frac{1}{4}L(F_2 - 2\sin\alpha \cdot \Delta T) \quad (24.1)$$

For the bilinear prestress and four-point bending load scheme:

$$M = \frac{1}{2}a(F_2 - 2\sin\alpha \cdot \Delta T) \quad (24.2)$$

For the trilinear prestress and three-point bending load scheme:

$$M = \frac{1}{4}F_2L - a \cdot \tan\beta \cdot \Delta T \quad (24.3)$$

For the trilinear prestress and four-point bending load scheme:

$$M = \frac{1}{2}a(F_2 - 2\tan\beta \cdot \Delta T) \quad (24.4)$$

For the same specimens, the M above should be equal to M_u calculated according to Eq. (23.1). Therefore, the load-bearing capacity F_2 in the second stage can be obtained. The total load-bearing capacity F of the prestressed composite beams can be calculated as follows:

$$F = F_1 + F_2 \quad (25)$$

5 Verification with Experiments

5.1 Deflection

5.1.1 Upward Deflection

The theoretical and experimental results of upward deflection are shown in Table 3. The results show that the theoretical values match well with the experimental values except for specimen B-30-I-1, indicating the accuracy of the theoretical analysis. The theoretical method cannot predict the upward deflection of specimens B-30-I-1, which may attribute to the measuring error of B-30-I-1 in the prestressing process.

Table 3 lists the upward deflection r and relative error δ_r without considering the slippage and shear deformation, and the average relative error is 4.67%. The calculation method is simple, and the accuracy is adequate for the engineered application. Meanwhile, the theoretical upward deflection R considering the additional deflection is also listed in Table 3. The results show that the upward deflection R is closer to the test values, the relative error δ_R is less than 5%, and the average error is only 2.03%, indicating that slippage and shear deformation are key factors in improving the theoretical analysis accuracy.

Table 3: Theoretical and test results of upward deflection

Specimens	f_{exp} (mm)	r (mm)	δ_r (%)	Δf_s (mm)	Δf_v (mm)	R (mm)	δ_R (%)
B-20-I-1	4.09	3.732	8.75	0.065	0.132	3.929	3.94
B-30-I-1	6.95	5.597	19.47	0.097	0.198	5.892	15.22
B-20-I-2	3.90	3.732	4.31	0.065	0.132	3.929	0.74
B-30-I-2	5.88	5.597	4.81	0.097	0.198	5.892	0.20
B-20-II-1	4.88	4.666	4.39	0.037	0.132	4.835	0.92
B-30-II-1	7.05	6.999	0.72	0.055	0.199	7.253	2.88
B-20-II-2	4.77	4.666	2.18	0.037	0.132	4.835	1.36
B-30-II-2	7.57	6.999	7.54	0.055	0.199	7.253	4.19

5.1.2 Midspan Deflection in Serviceability Limit State

According to standard [40], the maximum allowable deflection at the serviceability limit state is $l/250$ (l is the calculated span), which is equal to 14 mm in this paper. The corresponding load is defined as a serviceable load.

Due to the step load control strategy adopted in the test, the measuring equipment cannot exactly record the allowable deflection and serviceable load. Therefore, the deflection closed to 14 mm, and the corresponding load (P) was selected for comparison.

Fig. 11 compares the midspan deflection curves between the test and calculated during the loading process. The comparisons indicate that the theoretical method provides a feasible prediction of the deflection increase in the experiment.

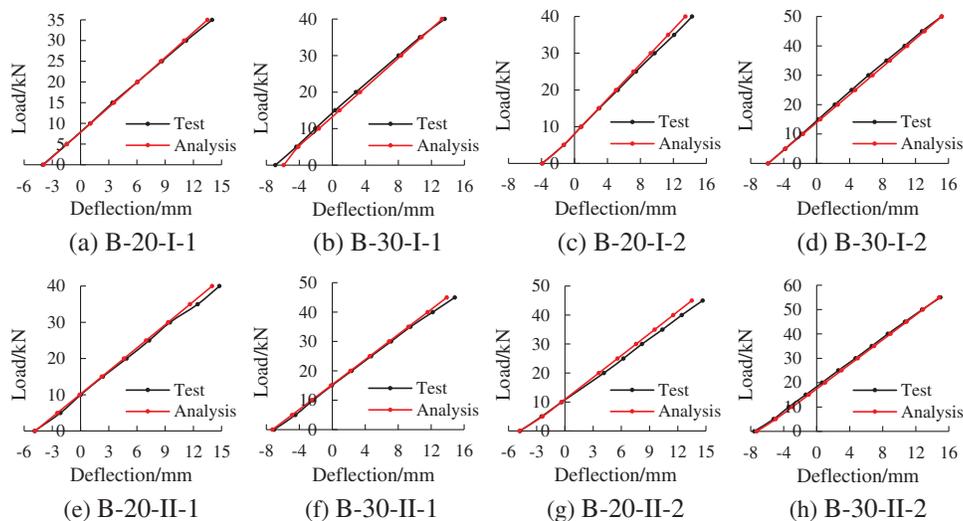


Figure 11: Comparisons of midspan deflections in serviceability limit state

Table 4 lists the theoretical and test results of midspan deflection in the serviceability limit state. It is noticed that the theoretical method (f_0) without considering the slippage and shear deformation causes larger errors; the average relative error is 12.41%. While for calculation results f_{s+v} considering the slippage and shear deformation, the accuracy is significantly improved, and the relative error δ_{s+v} is no more than 10%, the average errors reduce from 12.41% to 4.39%. It is shown that the theoretical calculation method based on slippage and shear deformation has higher accuracy, which can support the application of prestressed bamboo-steel composite beams.

Table 4: Theoretical and test results of midspan deflection

Specimen	P (kN)	f_{exp} (mm)	f_0 (mm)	δ_0 (%)	Δf_s (mm)	Δf_v (mm)	f_{s+v} (mm)	δ_{s+v} (%)
B-20-I-1	35	13.993	12.299	12.11	0.416	0.780	13.495	3.56
B-30-I-1	40	13.613	11.910	12.51	0.479	0.891	13.280	2.45
B-20-I-2	40	14.280	12.452	12.80	0.193	0.622	13.267	7.09
B-30-I-2	50	15.167	14.180	6.51	0.237	0.764	15.181	0.09
B-20-II-1	40	14.773	12.621	14.57	0.479	0.891	13.991	5.29
B-30-II-1	45	14.880	12.361	16.93	0.539	1.003	13.903	6.56
B-20-II-2	45	14.647	12.402	15.33	0.217	0.699	13.318	9.07
B-30-II-2	55	14.981	13.707	8.51	0.264	0.852	14.823	1.05

5.2 Bearing Capacity

The test results and calculation results of bearing capacity are listed in Table 5. The bearing capacity of composite beams in the serviceability limit state and ultimate limit state are calculated according to the theoretical analysis in Section 4. It is noticed that the calculation values F_{sc} in the serviceability limit state are in good agreement with the test value F_{se} . The relative error δ_s is no more than 10%, and the average error is 4.05%. While in the ultimate limit state, the theoretical method could also predict the test results except for specimen B-30-I-2, and the average relative error δ_u is 5.84%. For specimen B-30-I-2, due to the material defects, manufacture technology, and so on, the specimen was damaged in advance, resulting in a small bearing capacity and a considerable error.

Table 5: Theoretical and test results of bearing capacity

Specimen	Serviceability limit state			Ultimate limit state		
	F_{se} (kN)	F_{sc} (kN)	δ_s (%)	F_{ue} (kN)	F_{uc} (kN)	δ_u (%)
B-20-I-1	35.01	36.27	3.59	95	88.26	7.09
B-30-I-1	40.61	39.19	3.49	88	91.75	4.26
B-20-I-2	39.36	43.17	9.67	118	124.63	5.62
B-30-I-2	47.55	46.38	2.46	106	127.36	20.15
B-20-II-1	38.32	37.43	2.32	100	89.92	10.08
B-30-II-1	43.36	40.77	5.97	105	93.26	11.18
B-20-II-2	43.54	44.94	3.22	130	131.84	1.41
B-30-II-2	52.78	51.87	1.72	134	135.68	1.25

6 Conclusions

In this study, the theoretical analysis of deflection and bearing capacity of bamboo-steel composite beams is proposed and verified with experiment. The main results and findings are concluded as follows:

(1) The theoretical analysis of upward deflection is built based on the unit load method. The verification result indicates that the theoretical method considering slippage and shear deformation can improve the accuracy; the average errors reduce from 4.67% to 2.03%.

(2) A method is proposed to calculate the midspan deflection of composite beams. The comparisons indicate that the theoretical method provides a feasible prediction of the deflection in the experiment.

(3) The stress increment of prestressed reinforcement was established based on the force method. Then, a practical method is proposed to analyze the bearing capacity of the composite beams. The verification results show that the relative error of the proposed method is 4.05% and 5.85% for the serviceability limit state and ultimate limit state, which meet the accuracy requirement of the engineered application.

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