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Structural Interval Reliability Algorithm Based on Bernstein Polynomials and Evidence Theory

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Abstract: Structural reliability is an important method to measure the safety performance of structures under the influence of uncertain factors. Traditional structural reliability analysis methods often convert the limit state function to the polynomial form to measure whether the structure is invalid. The uncertain parameters mainly exist in the form of intervals. This method requires a lot of calculation and is often difficult to achieve efficiently. In order to solve this problem, this paper proposes an interval variable multivariate polynomial algorithm based on Bernstein polynomials and evidence theory to solve the structural reliability problem with cognitive uncertainty. Based on the non-probabilistic reliability index method, the extreme value of the limit state function is obtained using the properties of Bernstein polynomials, thus avoiding the need for a lot of sampling to solve the reliability analysis problem. The method is applied to numerical examples and engineering applications such as experiments, and the results show that the method has higher computational efficiency and accuracy than the traditional linear approximation method, especially for some reliability problems with higher nonlinearity. Moreover, this method can effectively improve the reliability of results and reduce the cost of calculation in practical engineering problems.

Keywords: Structural reliability; uncertainty analysis; interval problem; evidence theory; Bernstein polynomial

1 Introduction

Structural reliability is an essential tool to measure the safety performance of structures under the influence of uncertain factors. It is necessary to judge whether the reliability of traditional limit state calculation is reliable and the calculation cost is very high. In the existing methods, the deterministic mathematical model with safety coefficients is often adopted to complete the structural design. A complex deterministic model like the fractal fractional model [1] can be built by computer modeling, but it cannot accurately describe the uncertainty in engineering problems. Stochastic extension of the



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deterministic model [2] can address the above issue; however, various assumptions are imposed to simplify the calculation, which limits this method. To solve this problem, the method of fitting the probability density function of uncertain variables is often adopted. However, this method requires a lot of sampling, and the calculation cost sometimes exceeds that of the original method. Evidence theory [3] uses an interval composed of a pair of upper and lower probability values to describe the reliability of the problem and the uncertainty of the proposition. This interval contains the real probability value calculated by the traditional probability method. Thus, the problem of solving reliability is transformed into the problem of solving an interval value range [4]. Although it may cause conflicts in the evidence combination rule, some appropriate methods can avoid this problem [5]. Evidence theory uses probability bounds to reflect confidence in the collective power set of all possible outcomes. Evidence theory can deal with uncertain problems well in line with people's habits of thinking, which can reasonably describe and deal with all kinds of incomplete information, uncertain information, uncertain search is to find an effective, low-cost method for structural interval reliability.

In this paper, a method to solve the reliability probability interval by using Bernstein polynomials is proposed. The value range of limit state function is obtained by Bernstein polynomial approximation, and then the reliability interval of the problem is obtained based on evidence theory [8]. This method has higher efficiency and accuracy for implicit limit states and complex explicit expressions [9,10].

The rest of the paper is organized as follows: Section 2 briefly reviews some specific measures and open issues in the literature. Section 3 elaborates on our proposed method. Section 4 presents the experiments, and Section 5 describes the results and discussion. Section 6 concludes the paper.

2 Review of Typical Uncertainty Measures

In this section, we first introduce some basic concepts of evidence theory and then summarize its application model in structural reliability analysis. In addition, the current research status with the structural reliability analysis is illustrated by pieces of literature.

2.1 Basic Principles of Evidence Theory

Evidence theory was first proposed and developed by Dempster and Shafer, which is also called Dempster–Shafer theory. The Frame of Discernment (FD) is the most fundamental concept in evidence theory [11] which is the set of all possible results that can be recognized for a particular decision problem. The FD is similar to the sample of random variables in probability theory, which consists of a finite number of essential elements. As an essential concept of evidence theory, the Basic Probability Assignment (BPA) is used to describe the credibility of propositions. Θ is regarded as the FD if the set function (m: $2^x \rightarrow [0, 1]$ is the power set of Θ) satisfies the following three properties:

$$m(A) \ge 0, \forall A \in 2^{X}$$

$$m(\emptyset) = 0$$

$$\sum_{A \in \mathcal{X}} m(A) = 1$$
(1)

Then m is called the BPA on the FD of Θ . For all $A \in 2^X$, m(A) is called the primary credible number of A, where the set A of m(A) ≥ 0 is called the focal element of m. The primary credible number reflects the support of the evidence for the proposition that an element of X belongs to set A,

or the degree to which the decision-maker has reason to believe in proposition A under this evidence, which is similar to the probability density function in the random model [12–14]. The belief function (Bel) and plausibility function (Pl) represent the degree of truth of proposition A. Bel(A) is the sum of the basic credible numbers of evidence that fully support proposition A, and Pl(A) is the sum of the basic credible numbers that fully or partially support proposition A, that is:

$$\operatorname{Bel}(A) = \sum_{\substack{C \subseteq A \\ C \cap A \neq \varnothing}} m(C)$$
(2)

The two measures, one upper bound and the other lower bound, form an upper and lower probability interval to describe the uncertainty of proposition A [15,16].

2.2 Structural Reliability Analysis Based on Evidence Theory

Evidence theory uses an interval composed of a pair of upper and lower probability values to describe the reliability of the problem. This interval contains the true probability value P calculated by the traditional probability method. With the increased uncertain variable information, the upper probability Pl(A), lower probability Bel(A) and P will approach the same value [17]. For a single evidence variable, the above method can be used to convert the variable into an interval variable. For the case of multiple independent evidence variables, the Cartesian product method should be used to combine all evidence variables.

For a two-dimensional limit state equation g(a, b), $a \in A$ and $b \in B$ are two independent evidence variables. The reliability domain of the structure is defined as follows:

$$G = \{g : g(a,b) \ge g_0\}, (a,b) \in d_k, d_k = [a_i, b_j] \subset D$$
(3)

$$D = A \times B = \{ d_k = [a_i, b_j], a_i \in A, b_j \in B \}$$
(4)

where g_0 is the allowable response value, D is the Cartesian product of A and B, and a_i, b_j, d_k are the focal elements of A, B and D respectively. Multiple independent evidence variables are combined by the Cartesian product and shown as a multi-dimensional cube structure in geometry.

Based on the combination of BPA and reliability domain *G*, the reliability measures Bel(*G*) and Pl(*G*) of the structure can be calculated. These two measures will contain the probability reliability value $R = P\{g(a, b) \ge g_0\}$ under complete information:

$$\operatorname{Bel}(G) \le R \le \operatorname{Pl}(G) \tag{5}$$

Bel(G) and Pl(G) are obtained from Eq. (2), where set A is replaced by G, and C becomes the focal element of the *n*-dimensional FD:

$$\operatorname{Bel}(G) = \sum_{\substack{d_k \subseteq G \\ d_k \cap G \neq \varnothing}} \operatorname{m}_d(d_k)$$

$$\operatorname{Pl}(G) = \sum_{\substack{d_k \cap G \neq \varnothing}} \operatorname{m}_d(d_k)$$
(6)

The term $m_d(d_k)$ represents two joint BPAs. It can be seen that $d_k \subseteq G$ or $d_k \cap G \neq \emptyset$ is a necessary determiner when calculating Bel(G) and Pl(G); $d_k \subseteq G$ means that the focal element is entirely within the reliability domain, while $d_k \cap G \neq \emptyset$ indicates that the focal element is completely or partially

within the reliability domain. To judge accurately, it is necessary to calculate the extreme value of the limit state equation g on each focal element d_k :

$$[g_{\min}, g_{\max}] = \left[\min_{X \in d_k} g\left(X\right), \max_{X \in d_k} g\left(X\right)\right]$$
(7)

If both $g_{\min} - g_0$ and $g_{\max} - g_0$ are positive, it indicates $d_k \subseteq G$, and BPA is included in Bel(G) and Pl(G). If both $g_{\min} - g_0$ and $g_{\max} - g_0$ are negative, BPA is included in neither Bel(G) nor Pl(G). If $g_{\min} - g_0$ is negative but $g_{\max} - g_0$ is positive, it indicates $d_k \cap G \neq \emptyset$, and BPA is only included in Pl(G). By performing the above analysis on all focal elements, Bel(G) and Pl(G) can finally be obtained.

2.3 Related Works

In recent years, reliability research based on evidence theory has become a hot spot in the reliability research of high-tech equipment. The combination method proposed by Yang et al. [18] and Jiang et al. [19] considers multiple dimensions when fusing conflict evidence. However, there is still a problem that the consideration is not comprehensive or the probability subtraction method is used to measure the consistency of evidence at the decision-making level. In response to this problem, many experts and scholars have improved the evidence combination rules by modifying the evidence combination rules [20] and the actual evidence [21,22], respectively. However, the method of modifying the evidence combination rules fails to preserve the exchange laws and association law of the classical Dempster combination rules, which is not convenient for complex combination operations. Most of the previous methods to modify the actual evidence only consider the consistency of evidence at the reliability level. In contrast, the reliability evaluation depends on the probability distribution of indicators, so there is a lack of consistent measurement of evidence at the decision-making level. In literature [23], the conflict degree of evidence at the decision-making level is measured by the sum of the absolute values of the difference between the evidence and the Pignistic probability values of the same focal element, to define the correction weight of each evidence. However, the results of the consistency measurement between evidence by this method are often affected by the probability interval segmentation and are not accurate enough.

3 Methodology

In this method, the value range of the limit state function is obtained by Bernstein polynomial approximation, and then the reliability interval of the problem is obtained based on evidence theory. As seen from the above section, the key to reliability analysis of evidence theory is to solve the extreme value of the limit state function on each focal element [24]. There are two forms of limit state function for structural reliability analysis under cognitive uncertainty, namely, explicit limit state function and implicit limit state function. The explicit limit state function is divided into polynomial expressions and complex expressions. For complex expressions and implicit expressions, the response surface method is first used to establish the polynomial response surface approximation.

3.1 Response Surface Methodology

n

In this paper, the quadratic response surface method without cross terms is adopted. It approximates the actual limit state function g(x) in the following form:

$$g(\mathbf{x}) \approx G(\mathbf{x}) = a + \sum_{i=1}^{n} b_i x_i + \sum_{i=1}^{n} c_i x_i^2$$
(8)

where $\mathbf{x} = [x_1, x_2, ..., x_n]^T$ is an uncertain variable and $a, b_i, c_i (i = 1, ..., n)$ are 2n + 1 unknown coefficients to be determined, which can be obtained by fitting some test points. In this method, the first test point is taken as the mean point $\mathbf{x}_{\mu} = [\mu_1, \mu_2, ..., \mu_n]^T$ of uncertain variables, and the remaining 2n test points are taken as the maximum and minimum values of each uncertain variable. From this (2n + 1)th test point, the (2n + 1)th unknown coefficient can be obtained. Then the quadratic approximate limit state function without cross terms can be obtained.

3.2 Bernstein Polynomials for Extreme Values

After obtaining the approximate limit state function, the reliability analysis is transformed into the solution of the range of polynomials. Traditional optimization methods are complex in solving multidimensional and high-order problems, and there are optimal local problems. In this paper, a method based on Bernstein polynomials is proposed to obtain the value range of the limit state function. By using the properties of Bernstein polynomials [25], it is possible to solve Bernstein coefficients instead of finding the range directly.

Bernstein polynomials have the following properties for a given multivariate polynomial:

$$h\left(\boldsymbol{u}\right) = \sum_{0 \le i \le n} a_{i} \boldsymbol{u}^{i} \tag{9}$$

where $\mathbf{i} = [i_1, i_2, \dots, i_s]$, $\mathbf{u}^i = u_1^{i_1} \cdot u_2^{i_2} \cdot \dots \cdot u_s^{i_s}$ and $\mathbf{n} = [n_1, n_2, \dots, n_s]$, represent the highest values of each variable. If $u_1 \in [0, 1]$, $u_2 \in [0, 1]$, \dots , $u_s \in [0, 1]$ is satisfied, the original polynomial can be expanded into Bernstein polynomial form:

$$h(\boldsymbol{u}) = \sum_{0 \le i \le n} b_i(D,g) B_i^n(\boldsymbol{u})$$
(10)

and

$$\min\left(b_i\left(D,g\right)\right) \le h\left(u\right) \le \max\left(b_i\left(D,g\right)\right) \tag{11}$$

Among

$$B_{i}^{n}(\boldsymbol{u}) = B_{i_{1}}^{n_{1}}(u_{1}) \cdot B_{i_{2}}^{n_{2}}(u_{2}) \cdot \ldots \cdot B_{i_{s}}^{n_{s}}(u_{s}), \qquad (12)$$

$$B_{i_k}^{n_k}(u_k) = \binom{n_k}{i_k} (1 - u_k)^{n_k - i_k} u_k^{i_k}$$
(13)

is called the *i*th Bernstein term, and

$$b_i(D,g) = \sum_{j \le i} \prod_{k=1}^s \frac{\binom{i_k}{j_k}}{\binom{n_k}{j_k}} a_j$$
(14)

is called the term's Bernstein coefficient, where a_j corresponds to a_i in the original polynomial one-toone. Considering that $\binom{i}{j}$ is the probability of simply extracting *j* elements from *i* elements, the key to calculating the Bernstein coefficient is the coefficient a_i in the original polynomial.

Expanding the approximate polynomial of the limit state function into a Bernstein polynomial, the value range of the limit state function is obtained from the properties of the Bernstein polynomial

coefficients: $\min(b_i(D,g)) \le h(u) \le \max(b_i(D,g))$. The evidence theory is used to complete the reliability analysis.

3.3 Algorithm Design

Based on the above contents, the specific algorithm for interval reliability analysis based on Bernstein polynomials and evidence theory can be obtained, as shown in Fig. 1.



Figure 1: Algorithm flow chart

4 Example

4.1 Numerical Examples

For the composite beam shown in Fig. 2, the elastic modulus of the beam is E, the length, width, and height are L (mm), A (mm) and B (mm) respectively, and the elastic modulus of the aluminum plate on the lower surface of the beam is E_a , and the width and height of the effective section are C (mm) and D (mm) respectively. Six external vertical forces P_1 , P_2 , P_3 , P_4 , P_5 and P_6 are applied to six different positions of the beam, L_1 , L_2 , L_3 , L_4 , L_5 and L_6 respectively. The limit state function is defined as the difference between the allowable stress and the maximum stress:

$$Y = g(\mathbf{x}) = S - \sigma_{\max}(\mathbf{x}) \tag{15}$$



Figure 2: Composite beam diagram

Among them, S is the allowable stress, and $\sigma_{\max}(\mathbf{x})$ represents the maximum stress and its expression is:

$$\sigma_{\max}(\mathbf{x}) = \max\{\sigma_k(\mathbf{x}) : k = 1, \dots, 6\}$$
(16)

The stress $\sigma_k(x)$ applied to the cross section at P_k can be expressed as follows:

$$\sigma_{1}(\mathbf{x}) = \frac{[L_{1}\sum_{i=1}^{6}P_{i}(L-L_{i})/L]Y_{\max}(\mathbf{x})}{W(\mathbf{x})}$$
(17)

$$\sigma_k(\mathbf{x}) = \frac{\left[L_k \sum_{i=1}^6 P_i (L - L_i) / L - \sum_{i=1}^{k-1} P_i (L_k - L_i)\right] Y_{\max}(\mathbf{x})}{W(\mathbf{x})}, \quad k = (2, \dots, 6)$$
(18)

where P_i represents the external vertical force applied to the corresponding position of the beam, L_i is the length of the beam at this position, and

$$Y_{\max}(\mathbf{x}) = \frac{0.5AB^2 + DC(B+D)E_a/E_w}{AB + DCE_a/E_w}$$
(19)

$$W(\mathbf{x}) = \frac{AB^{3}}{12} + AB \left[Y_{\max}(\mathbf{x}) - \frac{B}{2} \right]^{2} + \frac{CD^{3}E_{a}}{12E_{w}} + \frac{CDE_{a}}{E_{w}} \left[\frac{D}{2} + B - Y_{\max}(\mathbf{x}) \right]^{2}$$
(20)

The values of the 15 fixed parameters in the model are: $L_1 = 200 \text{ mm}$, $L_2 = 400 \text{ mm}$, $L_3 = 600 \text{ mm}$, $L_4 = 800 \text{ mm}$, $L_5 = 1000 \text{ mm}$, $L_6 = 1200 \text{ mm}$, L = 1400 mm, $P_1 = P_2 = P_3 = P_4 = P_5 = P_6 = 15 \text{ kN}$, $E_a = 70 \text{ GPa}$ and $E_w = 8.75 \text{ GPa}$. Assume that variables *A*, *B*, *C* and *D* all obey normal distribution with mean values $\mu_A = 100 \text{ mm}$, $\mu_B = 200 \text{ mm}$, $\mu_C = 80 \text{ mm}$ and $\mu_D = 20 \text{ mm}$ and standard deviations $\sigma_A = 1 \text{ mm}$, $\sigma_B = 2 \text{ mm}$, $\sigma_C = 0.8 \text{ mm}$ and $\sigma_D = 0.2 \text{ mm}$. The BPA of each

variable is obtained through the probability model. Tables 1–3 show the BPA of each variable divided into four segments, eight segments and sixteen segments, respectively.

				, ,			
A/mm		В	/mm	C	/mm	D/mi	m
Interval	BPA	Interval	BPA	Interval	BPA	Interval	BPA
[95.0, 97.5]	0.62%	[190, 195]	0.62%	[78, 79]	0.62%	[19.50, 19.75]	0.62%
[97.5, 100]	49.38%	[195, 200]	49.38%	[79, 80]	49.38%	[19.75, 20.00]	49.38%
[100, 102.5]	49.38%	[200, 205]	49.38%	[80, 81]	49.38%	[20.00, 20.25]	49.38%
[102.5, 105]	0.62%	[205, 210]	0.62%	[81, 82]	0.62%	[20.25, 20.50]	0.62%

Table 1: Four intervals for A, B, C and D

Table 2: Eight intervals for A, B, C and D

A/mm		<i>B</i> /mm		C/mm		<i>D</i> /mm	
Interval	BPA	Interval	BPA	Interval	BPA	Interval	BPA
[95.0, 96.25]	0.1%	[190, 192.5]	0.1%	[78, 78.5]	0.1%	[19.50, 19.625]	0.1%
[96.25, 97.5]	0.61%	[192.5, 195]	0.61%	[78.5, 79]	0.61%	[19.625, 19.75]	0.61%
[97.5, 98.75]	9.94%	[195, 197.5]	9.94%	[79, 79.5]	9.94%	[19.75, 19.875]	9.94%
[98.75, 100]	39.44%	[197.5, 200]	39.44%	[79.5, 80]	39.44%	[19.875, 20.00]	39.44%
[100, 101.25]	39.44%	[200, 202.5]	39.44%	[80, 80.5]	39.44%	[20.00, 20.125]	39.44%
[101.25, 102.5]	9.94%	[202.5, 205]	9.94%	[80.5, 81]	9.94%	[20.125, 20.25]	9.94%
[102.5, 103.75]	0.61%	[205, 207.5]	0.61%	[81, 81.5]	0.61%	[20.25, 20.375]	0.61%
[103.75, 105]	0.1%	[207.5, 210]	0.1%	[81.5, 82]	0.1%	[20.375, 20.50]	0.1%

Table 3: Sixteen intervals for A, B, C and D

A/mm		<i>B</i> /mm		C/mm		<i>D</i> /mm	
Interval	BPA	Interval	BPA	Interval	BPA	Interval	BPA
[95.0, 95.625]	0.00%	[190, 191.25]	0.00%	[78, 78.25]	0.00%	[19.50, 19.56]	0.00%
[95.625, 96.25]	0.01%	[191.25, 192.5]	0.01%	[78.25, 78.5]	0.01%	[19.56, 19.625]	0.01%
[96.25, 96.875]	0.08%	[192.5, 193.75]	0.08%	[78.5, 78.75]	0.08%	[19.625, 19.69]	0.08%
[96.875, 97.5]	0.53%	[193.75, 195]	0.53%	[78.75, 79]	0.53%	[19.69, 19.75]	0.53%
[97.5, 98.125]	2.42%	[195, 196.25]	2.42%	[79, 79.25]	2.42%	[19.75, 19.81]	2.42%
[98.125, 98.75]	7.53%	[196.25, 197.5]	7.53%	[79.25, 79.5]	7.53%	[19.81, 19.875]	7.53%
[98.75, 99.375]	16.03%	[197.5, 198.75]	16.03%	[79.5, 79.75]	16.03%	[19.875, 19.94]	16.03%
[99.375, 100]	23.40%	[198.75, 200]	23.40%	[79.75, 80]	23.40%	[19.94, 20.00]	23.40%
[100, 100.625]	23.40%	[200, 201.25]	23.40%	[80, 80.25]	23.40%	[20.00, 20.06]	23.40%
[100.625, 01.25]	16.03%	[201.25, 202.5]	16.03%	[80.25, 80.5]	16.03%	[20.06, 20.125]	16.03%
[101.25, 01.875]	7.53%	[202.5, 203.75]	7.53%	[80.5, 80.75]	7.53%	[20.125, 20.19]	7.53%
[101.875, 102.5]	2.42%	[203.75, 205]	2.42%	[80.75, 81]	2.42%	[20.19, 20.25]	2.42%
[102.5, 103.13]	0.53%	[205, 206.25]	0.53%	[81, 81.25]	0.53%	[20.25, 20.31]	0.53%

(Continued)

Table 5. Continued								
A/mm	l	<i>B</i> /mr	n	C/m	m	D/mr	n	
Interval	BPA	Interval	BPA	Interval	BPA	Interval	BPA	
[103.13, 103.75] [103.75, 04.325]	0.08% 0.01%	[206.25, 207.5] [207.5, 208.75]	0.08% 0.01%	[81.25, 81.5] [81.5, 81.75]	0.08% 0.01%	[20.31, 20.375] [20.375, 20.44]	0.08%	
[104.325, 105]	0.00%	[208.75, 210]	0.00%	[81.75, 82]	0.00%	[20.44, 20.50]	0.00%	

 Table 3: Continued

Taking the allowable stress S = 0.0164, the Bernstein approximation method and linear approximation method proposed in this paper are used to analyze the reliability problem. They are compared with the safety results calculated directly by the genetic algorithm. The results are shown in Table 4. Case 1 represents the Bernstein approximation method, case 2 represents the linear approximation method, and case 3 represents the genetic algorithm. It can be seen from Table 4 that the interval probability obtained by dividing each variable into four segments, eight segments and sixteen segments by the method proposed in this paper is included in the safety probability interval, and the finer the number of variable segments, the smaller the interval probability span and the closer it is to the actual probability value (the real probability value is obtained by the Monte Carlo method, and the number of samples is 10°), which shows that the method proposed in this paper is effective. The proposed method has lower computational efficiency compared with the linear approximation method, but the results are closer to the actual value. It can be seen that the proposed method has higher accuracy, especially for reliability problems with high nonlinearity. At the same time, the computational efficiency of the proposed method and the linear approximation method is related to the number of evidence uncertainties. The less uncertain the quantification of evidence, the higher the computational efficiency, and vice versa. The calculation efficiency and accuracy are related to the interval number of variable divisions. The larger the interval number, the lower the efficiency and the higher the accuracy. Therefore, the method proposed in this paper is more suitable for the implicit limit state with less evidence uncertainty and the explicit complex expression. The appropriate interval score should be selected during the calculation.

Number of segments	Case	Bel	Pl	N*
4	Case 1	0.2509	0.7454	2304
	Case 2	0.2516	0.9908	1280
	Case 3	0.2523	0.9908	1280
8	Case 1	0.3043	0.6952	36864
	Case 2	0.3043	0.8638	20480
	Case 3	0.3047	0.8656	20480
16	Case 1	0.4289	0.6477	589824
	Case 2	0.4295	0.7646	327680
	Case 3	0.4486	0.7758	327680
	$MCS(10^{6})$		0.6072	

 Table 4: Interval probability calculation results

Note: *: N is the number of calls of the limit state function.

4.2 Engineering Examples

Fig. 3 shows the ten-bar structure. All horizontal and vertical bars are of length L, and $P_i(i = 1, 2, 3)$ is the concentrated load. The cross-sectional area and elastic modulus of each bar are $A_i(i = 1, 2, ..., 10)$ and E, respectively.



Figure 3: Schematic diagram of a ten-bar structure

Of these, the cross-sectional areas are $A_1 \in [6, 10] \text{ cm}^2$ for bar 1 and $A_2 \in [6, 10] \text{ cm}^2$ for bar 2, $L \in [1.3, 1.7]$ m is the evidence uncertainty, and the BPA structure is shown in Table 5. The rest are fixed parameters, and the values are: $A_{4-10} = 8 \text{ cm}^2$, E = 70 GPa, L = 1.5 m, $P_1 = 500 \text{ N}$ and $P_2 = P_3 = 100 \text{ N}$. The limit state equation can be defined as $g(x) = D - d_y(x)$, where $d_y(x)$ is the displacement in the vertical direction of node 3 and D is the allowable maximum displacement value, which is set as D = 0.03 m in this example.

$A_1, A_2,$	A_3/cm^2	<i>L</i> /m			
Interval	BPA	Interval	BPA		
[6, 7] [7, 8] [8, 9] [9, 10]	10.07% 39.93% 39.93% 10.07%	$[1.3, 1.4] \\ [1.4, 1.5] \\ [1.5, 1.6] \\ [1.6, 1.7]$	19.73% 30.27% 30.27% 19.73%		

Table 5: BPA structure of 4 intervals: A_1, A_2, A_3, L

5 Results and Discussion

To display the comparison results more clearly and accurately, the curves of Bel and Pl with the allowable stress S obtained by the two methods are given in this paper and compared with the actual probability curve obtained by the Monte Carlo method (the number of samples is 10^6). Figs. 4–6 show the results obtained by dividing the variables into four segments, eight segments and sixteen segments, respectively. As can be seen from the figure, in the whole process of S change, the Bel curve and Pl curve obtained by the two methods always include the actual probability P curve. With the increase of variable segmentation, i.e., the increase of information, the Bel curve and Pl curve gradually approach





Figure 5: 8-interval cumulative distribution results

0.017

0.015

Table 6 shows the obtained reliability analysis results. When the allowable maximum displacement D of node 3 is D = 60.0 mm, the plausibility Pl = 0.9821 is obtained by linear approximation. According to this result, structural safety is ideal. However, the plausibility obtained by the Bernstein approximation is Pl = 0.9317, which is relatively small, indicating an excellent possibility of structural failure under the design requirements of this displacement value. It can be seen that the accuracy of the proposed method is higher than that of the linear approximation method. When the maximum allowable displacement D of node 3 is D = 72.6 mm, the reliability and plausibility obtained by the two methods are 1. It can be seen that the safety index always meets the design needs; that is, the structure is entirely reliable.



Figure 6: 16-interval cumulative distribution results

Method	Case	Bel	Pl	D
Linear	60.0	0.5746	0.98211	60.0
approximation	72.6	1	1	72.6
Bernstein	60.0	0.5746	0.9317	60.0
	72.6	1	1	72.6

Table 6: Reliability analysis results

6 Conclusion

In this paper, a reliability analysis method based on evidence theory is proposed. Based on the non-probabilistic reliability index method, the extreme value of the limit state function is obtained by using the properties of the Bernstein polynomial to avoid large sampling.

The proposed method has been applied to a numerical example and an engineering application. The calculation results have shown that the proposed method has high calculation efficiency and accuracy, especially for some reliability problems with high nonlinearity. Compared with the linear approximation method, the proposed method in this paper has higher accuracy. Unfortunately, this study has not yet included large-scale reliability problems with many evidence uncertainties. It is expected that this method will be extended to the mixed reliability problem with cognitive uncertainty and random uncertainty in future.

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