Prediction of Compressive Strength of Self-Compacting Concrete Using Intelligent Computational Modeling

Susom Dutta¹, A. Ramachandra Murthy², Dookie Kim³ and Pijush Samui⁴

Abstract : In the present scenario, computational modeling has gained much importance for the prediction of the properties of concrete. This paper depicts that how computational intelligence can be applied for the prediction of compressive strength of Self Compacting Concrete (SCC). Three models, namely, Extreme Learning Machine (ELM), Adaptive Neuro Fuzzy Inference System (ANFIS) and Multi Adaptive Regression Spline (MARS) have been employed in the present study for the prediction of compressive strength of self compacting concrete. The contents of cement (c), sand (s), coarse aggregate (a), fly ash (f), water/powder (w/p) ratio and superplasticizer (sp) dosage have been taken as inputs and 28 days compressive strength (f_{ck}) as output for ELM, ANFIS and MARS models. A relatively large set of data including 80 normalized data available in the literature has been taken for the study. A comparison is made between the results obtained from all the above-mentioned models and the model which provides best fit is established. The experimental results demonstrate that proposed models are robust for determination of compressive strength of self-compacting concrete.

Keywords: Self Compacting Concrete (SCC), Compressive Strength, Extreme Learning Machine (ELM), Adaptive Neuro Fuzzy Inference System (ANFIS), Multi Adaptive Regression Spline (MARS).

1 Introduction

Concrete is composed mainly of cement (commonly Portland cement), fine aggregate, coarse aggregate and water. Concrete is a versatile material that can easily be mixed to meet a variety of special needs and formed to virtually any shape. Concrete solidifies and hardens after mixing with water and placement due to a chemical process known as hydration. The water reacts with the cement, which bonds the other components together, eventually creating a stone-like material. The uniaxial compressive strength of concrete is considered as the most crucial property in case of concrete mix design and quality control

¹ Undergraduate Student, School of Mechanical & Building Sciences (SMBS), VIT University, Vellore, Tamil Nadu 632014, India. Email: susomdutta7@gmail.com.

² Senior Scientist, Computational Structural Mechanics Group, CSIR-Structural Engineering Research Centre, Taramani, Chennai-600 113. Email : murthyarc@serc.res.in

³ Professor, Department of Civil Engineering, Kunsan National University, Kunsan, Jeonbuk, South Korea. Email: kim2kie@chol.com

⁴ Associate Professor, National Institute of Technology, Patna, India, Email: pijush@nitp.ac.in.

which is determined by number of factors. Several factors affect the concrete mix design like to derive a concrete as High-Performance Concrete, it should possess, in addition to good strength, several other favorable properties. The water/cement (w/c) ratio in the concrete is lower than normal concrete which requires special additives in the concrete, along with a superplasticizer to obtain good workability.

The nature of aggregate is important to incur high strength. The gradation of the aggregates influences the workability. The order in which the materials are mixed is also important for the workability of the concrete. From engineering point of view, strength is the most important property of structural concrete. The strength of the concrete is determined by the characteristics of the mortar, coarse aggregate, fine aggregate and the interface. Property of concrete is influenced by the properties of each constituent added in it. For example, for the same quality mortar, diverse types of coarse aggregate with different shape, texture, mineralogy, and strength may result in different concrete strengths. The tests for compressive strength are generally carried out at about 7 or 28 days from the day when the concrete is casted. Generally, strength after 28-days is standard and therefore essential and if required strength for other ages can be carried out. Accidentally, if there is some experimental error in designing the mix, the test results will fall short of required strength, the entire process of concrete design must be repeated which may be a costly and time consuming. The same applies to all types of concrete, i.e. normal concrete, self-compacting concrete, ready mixed concrete, etc. It is well acknowledged that prediction of the compressive strength of concrete is most important in modern concrete designing and in taking engineering decisions.

The property of a self-compacting concrete (SCC) [Schutter et al (2008)] is the fresh concrete should flow around reinforcement and consolidate within formwork under its own weight that exhibits no defects due to segregation or bleeding. The guiding principle for this type of concrete is that the sedimentation velocity of a particle is inversely proportional to the viscosity of the floating medium in which the particle exists. The mix design principle is that the flowability and viscosity of the paste is adjusted by proportioning the cement and additives, water to powder ratio and then by adding superplasticizers and Viscosity Modifying Admixtures (VMA). It requires manipulation of several mixture variables to ensure satisfactory flowable behavior and proper mechanical properties. Also, absence of theoretical relationships between mixture proportioning and measured engineering properties of SCC makes it more complex.

This study adopts Extreme Learning Machine (ELM), Adaptive Neuro Fuzzy Inference System (ANFIS), Multivariate Adaptive Regression Spline (MARS) for prediction of 28 days compressive strength of Self Compacting Concrete (SCC). ELM proposed by Huang et al. (2004), is an easy-to use and effective learning algorithm of single-hidden layer feed-forward neural networks (SLFNs). The classical learning algorithm in neural network, e. g. backpropagation, requires setting several user-defined parameters and may get into local minimum. ELM is used in various fields like Renewable Energy [Wang et al. (2015)], Neurocomputing [Fu A-M. et al. (2014)], Mechanical Engineering [Gao et al. (2013)], Bioinformatics [Priya et al. (2012)]. ANFIS [Takagi and Sugeno (1985)] can be trained to provide input/output data mappings and one can get the relationship between model inputs and corresponding outputs. ANFIS is a kind of artificial neural network that

is based on Takagi-Sugeno fuzzy inference system. It enables the knowledge that has been learnt in the network training to be translated into a set of fuzzy rules that describe the model input/output relationship in a more transparent fashion. It is employed in many fields such as Powder Technology [Pourtousi et al. (2015)], Applied Energy [Yang and Entchev (2014)], Hydrology [Chang and Wang (2013)], Communications [Lee et al. (2012)]. MARS is a flexible, more accurate, and faster simulation method for both regression and classification problems [Friedman (1991); Salford Systems (2001)]. It is capable of fitting complex, nonlinear relationships between output and input variables. Some examples of its usage are Biological conversations [Kandel et al. (2015)], Ecological Modeling [Pickens and King (2014)], Transportation [Sun et al. (2013)]. The data used in both these techniques is taken from Siddique et al. (2011).

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2 Dataset employed

The data used (Table 1) in both the techniques are normalized against their maximum values [Siddique et al. (2011)]. In carrying out the formulation, the data has been divided into two sub-sets:

(a) Training dataset: This is required to construct the model. In this study, 64 (80% of total data) out of the 80 values are considered as training dataset.

(b) A testing dataset: This is required to estimate the model performance. In this study, the remaining 16 (20% of total data) values are considered as testing dataset.

Sr. No.	Cement	Fly ash	Water/powder	SP dosage	Sand	Coarse Agg	Comp. Strength
	(kg/m3)	(kg/m3)		(%)	(kg/m3)	(kg/m3)	(MPa)
1	290	100	0.45	0.8	913	837	42.7
2	250	261	0.55	0.5	478	837	17
3	210	100	0.65	0.8	910	837	19.1
4	250	160	0.55	0.5	742	837	24.1
5	210	220	0.45	0.8	768	837	26.7
6	290	100	0.65	0.2	709	837	26.6
7	290	220	0.45	0.2	625	837	32.9
8	250	160	0.55	0.5	742	837	26
9	250	160	0.55	0.5	742	837	28.5
10	250	160	0.55	0.5	742	837	26.4
11	250	160	0.55	0	739	837	27.3
12	317	160	0.55	0.5	594	837	29.1
13	210	220	0.65	0.2	562	837	10.2
14	250	160	0.55	0.5	742	837	25.3
15	250	160	0.38	0.5	919	837	36.3

Table 1: Details of the data for prediction of compressive strength of SCC

16	250	160	0.55	1	746	837	26.7
17	250	160	0.72	0.5	566	837	11
18	183	160	0.55	0.5	891	837	22.1
19	220	180	0.39	0.35	916	900	49
20	220	180	0.39	0.35	916	900	49
21	160	240	0.39	0.35	886	900	44
22	193	158	0.39	0.35	1024	900	44
23	220	180	0.45	0.35	850	900	38
24	198	232	0.34	0.2	874	900	46
25	248	203	0.39	0.35	808	900	50
26	237	133	0.36	0.2	1034	900	49
27	220	180	0.39	0.35	916	900	49
28	237	133	0.43	0.5	960	900	46
29	275	155	0.43	0.5	827	900	48
30	280	120	0.39	0.35	946	900	45
31	170	200	0.43	0.2	930	900	31
32	220	180	0.39	0.6	916	900	43
33	220	180	0.39	0.35	916	900	47
34	220	180	0.39	0.1	916	900	44
35	198	232	0.36	0.5	872	900	52
36	220	180	0.39	0.35	916	900	45
37	220	180	0.33	0.35	982	900	51
38	170	200	0.43	0.5	928	900	33
39	275	155	0.43	0.2	830	900	36
40	247	165	0.45	0.12	845	846	34.6
41	238	159	0.4	0.29	844	844	37.8
42	232	155	0.35	0.38	846	847	48.3
43	207	207	0.45	0.4	845	843	33.2
44	200	200	0.4	0.17	842	843	34.9
45	197	197	0.35	0.28	856	856	38.9
46	169	254	0.45	0	853	853	30.2
47	163	245	0.4	0.2	851	851	26.2
48	161	241	0.35	0.3	866	864	35.8
49	350	162	0.59	0.09	768	840	51.7
50	349	162	0.57	0.14	779	852	59.9
51	350	133	0.52	0.16	815	883	55.3
52	350	111	0.51	0.15	831	900	61

53	250	257	0.77	0.11	787	853	51.5
54	427	115	0.45	0.12	779	844	59.4
55	348	224	0.5	0.43	783	848	58.6
56	350	90	0.48	0.14	852	923	46.5
57	327	173	0.53	0.2	902	803	61.6
58	380	145	0.48	0.1	788	854	73.5
59	350	186	0.51	0.11	786	851	70.4
60	380	145	0.48	0.13	988	659	65.5
61	380	192	0.53	0.1	931	621	67.8
62	275	250	0.67	0.09	775	840	54.5
63	325	60	0.65	0.43	899	850	30.8
64	325	60	0.65	0.43	899	850	32.6
65	325	120	0.75	0.43	755	850	32.2
66	249	60	0.68	0.43	1079	850	24
67	325	60	0.85	0.43	722	850	13.3
68	370	96	0.57	0.25	833	850	39.5
69	400	60	0.63	0.43	718	850	30.4
70	325	60	0.65	0.43	899	850	35.3
71	370	24	0.69	0.62	770	850	18.7
72	325	0	0.55	0.43	1042	850	41.2
73	280	96	0.87	0.25	820	850	19.6
74	325	60	0.65	0.75	896	850	27.7
75	325	60	0.65	0.43	898	850	35
76	325	60	0.65	0.12	900	850	31.4
77	370	96	0.57	0.62	830	850	38.8
78	325	60	0.65	0.43	898	850	34.3
79	280	96	0.87	0.62	817	850	15.9
80	370	24	0.69	0.25	772	850	26.4

3 Extreme learning machine (ELM) model

The ELM algorithm was originally proposed by Huang *et al.* in 2004 and it makes use of the SLFN. The main concept behind the ELM lies in the random initialization of the SLFN weights and biases. Then, using Theorem 1 and under the conditions of the theorem, the input weights and biases do not need to be adjusted and it is possible to calculate implicitly the hidden-layer output matrix and hence the output weights. The network is obtained with very few steps and very low computational cost.

The defects of gradient-based learning in a single hidden-layer feedforward neural network (SLFN) are avoided by using ELM. It determines optimal weights analytically.

Let us consider the following two datasets.

$$D_{training} = \{x_{1i}, y_{1i}\}_{i=1}^{N}$$
(1)
$$D_{testing} = \{x_{2i}, y_{2i}\}_{i=1}^{N}$$
(2)

$$Diesting = (x_{2i}, y_{2i})_{i=1}$$

Where x is the input, y is the output and N is number of datasets.

In this paper,

$$x = Normalized[c, s, a, f, w/p, sp]$$
$$y = Normalized[f_{ck}].$$

In a single hidden layer feed forward networks (SLFN), the relation between input and output is given below:

$$\sum_{i=1}^{N} \beta_{i} f(w_{i} \cdot x_{j} + b_{i}) = y_{ji}, j = 1,..N.$$
(3)

where w_i is the input weight vector between the ith neuron in the hidden layer and the input layer, b_i means the input bias of the ith neuron in the hidden layer, x_j is the jth input data vector, f() is an activation function of the hidden neuron, β_i is the output weight vector between the ith hidden neuron and the output layer, \overline{N} is number of hidden nodes, N is number of training samples and y_j means the target vector of the jth input data.

Equation (3) can be written in the following way.

$$H\beta = T \tag{4}$$

$$H = \begin{bmatrix} g(w_1.x_1 + b) & \cdots & g(w_{\overline{N}}.x_1 + b_{\overline{N}}) \\ \vdots & \cdots & \vdots \\ g(w_1.x_N + b) & \cdots & g(w_{\overline{N}}.x_N + b_{\overline{N}}) \end{bmatrix}_{N \times \overline{N}}, \beta = \begin{bmatrix} \beta_1^T \\ \vdots \\ \beta_{\overline{N}}^T \end{bmatrix}_{\overline{N} \times m} \text{ and } T = \begin{bmatrix} y_1^T \\ \vdots \\ y_N^T \end{bmatrix}_{N \times m}.$$
(5)

where H is the hidden layer output matrix of the network (Huang and Babri, 1998; Huang, 2003).

In ELM, the values of w_i and b_i are not tuned during training. Random values are assigned for w_i and b_i according to any continuous sampling distribution (Huang et al., 2004; Huang and Siew, 2004, 2005). The value of β is determined from the following equation.

$$\beta = H^{-1}T \tag{6}$$

Where H^{-1} is the Moore-Penrose generalized inverse [Serre (2002)] of the hidden layer output matrix H.

ELM has been developed by using MATLAB (MathWork Inc R2012a).

4 Adaptive neuro fuzzy inference system (ANFIS) model

Fuzzy logic is a form of many-valued logic and it deals with reasoning that is approximate rather than fixed and exact. The nature of uncertainty in a slope design is very important that should be considered. Fuzzy set theory was developed specially to deal with uncertainties that are nonrandom in nature.

There are several FISs that have been employed in various applications. The most commonly used include:

- Mamdani Fuzzy Model;
- Takagi-Sugeno-Kang fuzzy (TSK) model;
- Tsukamoto fuzzy model;
- Singleton fuzzy model.

In fuzzy system, all sets are not crisp, but some are fuzzy. These fuzzy sets can be modeled in linguistic human terms such as large, small and medium [Takagi and Sugeno (1985)]. This is very valuable to model human behavior. A fuzzy set is a set containing elements that have varying degree of membership. The degree of membership gives fuzzy sets flexibility in modeling [Bezdek (1981)]. The membership can be discrete or continuous type. The most commonly used membership functions are triangular, trapezoidal, gaussian and bell function. ANFIS makes inference by fuzzy logic and shapes fuzzy membership function using neural network [Altrock (1995); Brown and Harris (1995)]. In the literature, there are several inference techniques developed for fuzzy rule-based systems, such as Mamdani and Sugeno [Brown and Harris (1995)]. In this study, Sugeno-type systems have been used. In Sugeno, output of the fuzzy rule is differentiated by a crisp function. Typical representation of a fuzzy rule in Sugeno system is given by: if x_1 is A_1 and x_2 is A_2 ...and x_N is A_N then y=f(x), where A_1, A_2 ... and A_N are fuzzy sets and y is crisp function. In this system, outcome of each rule is crisp value and weighted average has been used to calculate the result of all the rules. The definition of the nonlinear mapping of Sugeno-type system (f_{FS}) can be given as follows:

$$f_{FS} = \frac{\sum_{i=1}^{m} w_i \prod_{j=1}^{n} \mu_{A_j^i}(x_j)}{\sum_{i=1}^{m} \prod_{j=1}^{n} \mu_{A_j^i}(x_j)}$$
(7)

In which *m* is the number of rules, *n* is the number of data points and μ_A is the membership function of fuzzy set A. membership function has been determined iteratively to produce correct outputs by ANFIS. There are different types of membership functions such as triangular, trapezoidal, gaussian and bell function. In this analysis, Gaussian membership function has been used. The form of the gaussian function used is as follows:

$$f(x,\sigma,c) = e^{\frac{-(x-c)^2}{2\sigma^2}}$$
(8)

where, *c* and is the mean and standard deviation of the data respectively. Learning process in ANFIS methodology is commonly performed by two techniques, i.e. back propagation and hybrid learning algorithms.

ANFIS has been developed by using MATLAB (MathWork Inc R2012a).

5 Multi adaptive regression spline (MARS) model

MARS is widely accepted by researchers and practitioners for the following reasons.

- MARS is capable of modeling complex non-linear relationship among variables without strong model assumptions.
- MARS can capture the relative importance of independent variables to the dependent variable when many potential independent variables are considered.
- MARS does not need long training process and hence can save lots of model building time, especially when the dataset is huge.

Finally, one strong advantage of MARS over other classification techniques is the resulting model can be easily interpreted. It not only points out which variables are important in classifying objects/observations, but also indicates an object/observation belongs to a specific class when the built rules are satisfied. The final fact has important managerial and interpretative implications and can help to make appropriate decisions.

The MARS model splits the data into several splines on an equivalent interval basis (Friedman 1991). In every spline, MARS splits the data further into many subgroups (Yang *et al.*, 2003). Several knots are created by MARS. These knots can be located between different input variables or different intervals in the same input variable, to separate the subgroups. The data of each subgroup are represented by a basis function (BF). The general form of a MARS predictor is as follows:

$$f(x) = \beta_0 + \sum_{j=1}^{P} \sum_{b=1}^{B} \left[\beta_{jb}(+) Max(0, x_j - H_{bj}) + \beta_{jb}(-) Max(0, H_{bj} - x_j) \right]$$
(9)

where x=input, f(x) =output, P= predictor variables and B=basis function. Max (0, x-H) and Max (0, H-x) are BF and do not have to each be present if their coefficients are 0. The H values are called knots. The spline function consists of two segments, i.e. truncated functions of the left-hand side of Equation (10) and right-hand side Equation (11) separated from each other by a so-called knot location [Veaux *et al.*, 1993], as follows:

$$b_{q}^{-}(x-t) = \left[-(x-t)\right]_{+}^{q} = \begin{cases} (t-x)^{q} & \text{if } x > t \\ 0 & \text{otherwise} \end{cases}$$
(10)

$$b_q^+(x-t) = \left[+ (x-t) \right]_+^q = \begin{cases} (x-t)^q & \text{if } x > t \\ 0 & \text{otherwise} \end{cases}$$
(11)

...

where: t is the knot location and $b_q^-(x-t) \& b_q^+(x-t)$ are the spline functions. The MARS algorithm consists of (i) a forward stepwise algorithm to select certain spline basis functions, (ii) a backward stepwise algorithm to delete BFs until the "best" set is found, and (iii) a smoothing method which gives the final MARS approximation a certain degree of continuity. BFs are deleted in the order of least contributions using the generalized cross-validation (GCV) criterion (Craven & Wahba 1979). The GCV criterion is defined in the following way

$$GCV = \frac{\frac{1}{N} \sum_{i=1}^{N} [y_i - f(x_i)]^2}{\left[1 - \frac{C(B)}{N}\right]^2}$$
(12)

where N is the number of data and C(B) is a complexity penalty that increases with the

number of BF in the model and which is defined as:

$$C(B) = (B+1) + \lambda B \tag{13}$$

where is a penalty for each BF included into the model, it can be also regarded as a smoothing parameter. [Friedman 1991] provided more details about the selection of the MARS has been developed by using MATLAB (MathWork Inc R2012a).

6 Result and discussions

Error and Correlation Calculations

The validity of each model can be verified using these following formulas:

The mean absolute error (MAE) is a quantity used to measure how close predictions are to the actual value.

$$MAE = \frac{\sum_{i=1}^{n} |w_{ai} - w_{pi}|}{n}$$
(14)

Root-mean-square error (RMSE) is used to measure the differences between predicted value by the models and the actual values.

$$RMSE = \sqrt{\frac{\sum_{i=1}^{n} (w_{ai} - w_{pi})^{2}}{n}}$$
(15)

Coefficient of correlation(R) has been used as main criterion to examine the performance of the developed models. The value of R has been determined by using the following equation:

$$R = \frac{\sum_{i=1}^{n} \left(w_{ai} - \overline{w}_{a} \right) \left(w_{pi} - \overline{w}_{p} \right)}{\sqrt{\sum_{i=1}^{n} \left(w_{ai} - \overline{w}_{a} \right)} \sqrt{\sqrt{\sum_{i=1}^{n} \left(w_{pi} - \overline{w}_{p} \right)}}}$$
(16)

 ρ is known as the Performance Index is used to check the accuracy of the predicted values.

$$\rho = \frac{RMSE}{\overline{w}_a} \frac{1}{R+1} \tag{17}$$

where w_{ai} and w_{pi} are the actual and predicted W values, respectively, \overline{w}_a and \overline{w}_p are mean of actual and predicted W values corresponding to n patterns. For a predictive model of high accuracy, the value of R should be close to one.

For developing ELM, the numbers of hidden nodes have consequences on training performance. The best performance is obtained at 15 hidden nodes. Therefore, the number of hidden nodes is set to 15. The initial training datasets is assigned as 64. The Block Range is provided as 25. Radial basis function has been adopted as activation function. Graphs are plotted between Actual Normalized Strength and Predicted Normalized Strength. Figure 1 shows the performance of training and testing dataset respectively. After the compilation of the model, following results are obtained.

Training and testing performance are illustrated in the table 4.

As shown in figure 1, the value of R is nearly close to one for training as well as testing datasets. Therefore, the developed ELM proves that it is quite capable of predicting of 28 days compressive strength of SCC.

The value of error and correlation functions for ELM is shown in table 4.



Figure 1: Performance of training and testing dataset (ELM)

In ANFIS model, Gaussian membership function has been used in this analysis. The hypothesized initial numbers of membership functions for each input are 55. A suitable pattern has to be chosen for the best performance of the network. Figure 2 shows the architecture of ANFIS model for this study.



Figure 2: Architecture of ANFIS model

After the training (with 50 epochs) was complete, the final configuration for the Fuzzy Inference System (FIS) is:

Number of output membership functions	55
Number of fuzzy rules	55

Neuro-fuzzy adaptive network for Strength:

Number of input	6 (c, s, a, f, w/p and sp)
Number of membership function for each input	55
Type of membership functions for each input	Gaussian
Type of membership functions for each output	Linear
Number of training epochs	50

Training and testing performance are illustrated in the table 4.

The performance of training and testing dataset has been illustrated in figure 2. It is observed from figure that the value of R is equal to one for training as well as testing dataset. So, this proves that it is the most effective technique/model for the prediction of 28 days compressive strength of SCC.

The value of error and correlation functions for ANFIS is shown in table 4.



Figure 3: Performance of training and testing dataset (ANFIS)

For MARS model, during training, the forward stepwise procedure was carried out to select 42 basis functions (BF) to build the MARS model. This was followed by the backward stepwise procedure to remove redundant basis functions. The final model includes 36 basis functions, which are listed in Table 2 together with their corresponding equations and a_m .

Basis Function	Equation	a _m
BF1	Max (0, c -0.337078651685393)	0.4165
BF2	Max (0, 0.337078651685393 -c)	-0.5637
BF3	Max (0, a -0.444444444444444444444444444444444444	-0.0704
BF4	BF3 * max (0, 0.43 -f)	-0.0574
BF5	BF1 * max (0, 0.3333333333333333333333333333333)	1.5582
BF6	Max (0, w/p -0.512479201331115)	1.1826
BF7	Max (0, 0.512479201331115 –w/p)	-0.9909
BF8	Max (0, 0.44444444444444 -a) * max (0, sp - 0.923841059602649)	-17.9948
BF9	Max (0, 0.44444444444444 -a) * max (0, 0.923841059602649 -sp)	0.0267
BF10	Max (0, s -0.593869731800766)	0.8714
BF11	BF9 * max (0, s -0.55555555555555556)	2.0546
BF12	BF9 * max (0, 0.555555555555556 -s)	-69.6125

Table 2: list of basis functions which give the best performance

BF13	BF1 * max (0, f -0.43)	23.4321
BF14	BF1 * max (0, 0.43 -f)	2.9236
BF15	BF6 * max (0, f -0.35)	-5.7696
BF16	BF2 * max (0, s -0.632183908045977)	-1.8302
BF17	BF2 * max (0, 0.632183908045977 -s)	101.1253
BF18	BF17 * max (0, sp -0.748344370860927)	-506.0321
BF19	BF17 * max (0, 0.748344370860927 -sp)	-2881.9644
BF20	BF9 * max (0, 0.3 -f)	-1.9861
BF21	BF6 * max (0, c -0.224719101123596)	-0.8562
BF22	BF6 * max (0, 0.224719101123596 -c)	-3.9773
BF23	BF1 * max (0, sp -0.758278145695364)	1.2984
BF24	BF1 * max (0, 0.758278145695364 -sp)	-10.7550
BF25	Max (0, sp -0.735099337748344)	-0.1593
BF26	Max (0, 0.735099337748344 -sp)	5.0107
BF27	BF25 * max (0, f -0.2)	6.1196
BF28	BF25 * max (0, 0.2 -f)	4.4944
BF29	BF25 * max (0, s -0.689655172413793)	4.9905
BF30	BF25 * max (0, 0.689655172413793 -s)	-2.4291
BF31	BF11 * max (0, f -0.3)	4.4487
BF32	BF11 * max (0, 0.3 -f)	-17.1001
BF33	BF29 * max (0, 0.2 -f)	-3.0855
BF34	BF27 * max (0, a -0.11111111111111111)	-17.1058
BF35	BF27 * max (0, 0.111111111111111111111)	-28.8211

The final equation for the prediction of strength (f_{ck}) based on MARS model is given below:

$$f_{ck} = 0.2947 + \sum_{m=1}^{M} a_m B_m(x)$$
(18)

where,

 $a_0=0.2947 = \text{coefficient}$ of the constant basis function, or the constant term; $\{a_m\} = \text{vector of coefficients of the non-constant basis functions}, m=1, 2, ..., M;$ B_m are the basis functions that are selected for inclusion in the model; The ANOVA decomposition is specified in row wise for each ANOVA function. The columns represent summary quantities for corresponding ones. The first column lists the function number. The second gives the standard deviation (STD) of the function. This gives indication of its (relative) importance to the overall model and can be interpreted in a manner like a standard regression coefficient in a linear model. The third column provides another indication of the importance of the corresponding ANOVA function, by listing the GCV score for a model with the entire basis functions corresponding to that ANOVA function removed. This can be used to judge whether this ANOVA function is making an important contribution to the model, or whether it just slightly helps to improve the global GCV score. The fourth column gives the number of basis functions comprising the ANOVA and the last column of Table 3 gives the predictor variables associated with the ANOVA function. Table 3 shows the ANOVA decomposition for Training dataset.

Function Number	Standard Deviation	GCV	Basis Function	Parameters	Variable(s)
1	0.112	0.059	2	2	1
2	0.108	0.1	1	1	2
3	0.005	0.003	1	1	3
4	0.197	0.225	2	2	5
5	0.596	1.645	2	2	6
6	0.508	1.174	2	2	12
7	0.017	0.005	1	1	13
8	0.197	0.222	2	2	14
9	0.048	0.058	2	2	15
10	0.643	1.927	2	2	16
11	0.062	0.021	2	2	26
12	0.001	0.003	1	1	34
13	0.041	0.014	2	2	36
14	0.11	0.077	1	1	4 5
15	0.116	0.08	2	2	4 6
16	0.471	1.019	2	2	126
17	0.081	0.033	2	2	236
18	0.001	0.003	1	1	246
19	0.032	0.008	3	3	346
20	0.013	0.004	2	2	2346

 Table 3: ANOVA decomposition for Training dataset

Figure 3 depicts the performance of training and testing dataset. It is observed from figure that the value of R is close to one for training but not close to one for testing datasets. Therefore, the developed MARS proves its feeble ability for prediction of 28 days compressive strength of SCC.

The value of error and correlation functions for MARS is shown in table 3.



Figure 4: Performance of training and testing dataset (MARS)

The approximates of error and correlation functions i.e. mean absolute error (MAE), rootmean-square error (RMSE), coefficient of correlation(R) and performance index (ρ) for all the methods employed are consolidated in table 4.

Models Mean absolute Employederror (MAE)			Root-mea error (RN	n-square ISE)	Coefficient of Performanc correlation(R) (ρ)			nce Index
	Training	Testing	Training	Testing	Training	Testing	Training	Testing
ELM	0.05842	0.06058	0.07457	0.08010	0.9419	0.8949	0.07918	0.14248
ANFIS	0.00361	0.00092	0.00985	0.00211	0.9990	0.9999	0.01016	0.00356
MARS	0.02106	0.09222	0.02582	0.11046	0.9932	0.8068	0.02671	0.20605

Table 4: Approximates of error and correlation functions

A comparative study has been carried out between the developed ELM, ANFIS and MARS models. Figure 1, 2 and 3 shows the graph of R value of the training and testing datasets for ELM, ANFIS and MARS models respectively. It can be inferred from figure 2 that the performance of ANFIS is best than the performance of ELM and MARS model. It is also clear from Table 5 that the performance of ANFIS is best.

The performance of training and testing dataset is almost same for the ELM and MARS models but ANFIS shows the best performance among the three models. So, the developed models do not show overtraining. Therefore, the developed models have good generalization capability. Datasets are normalized between for developing the ELM, ANFIS and MARS models. The developed models do not make assumption about the dataset. The developed MARS gives equation for prediction of strength. However, ANFIS and MARS do not use statistical parameters of the dataset for prediction. ELM makes use of single-hidden layer feed-forward neural networks (SLFNs). ANFIS uses membership function for developing the model. MARS adopts basis function for final prediction.

7 Summary and conclusions

This study has described the application of ELM, ANFIS and MARS models for the prediction of 28 days compressive strength of Self Compacting Concrete (SCC). The performance of ANFIS is better than ELM and MARS model. User can employ the developed model for prediction of compressive strength. The developed models can be used as a quick tool for prediction of 28 days compressive strength of Self Compacting Concrete (SCC). This paper shows that the developed ANFIS is a robust model for prediction of 28 days compressive strength of Self Compacting Concrete (SCC).

References

Altrock, C. V. (1995): *Fuzzy logic and neuro-fuzzy applications explained*, Prentice-Hall, New Jersey.

Bezdek, J. C. (1981): Pattern recognition with fuzzy objective function algorithms, Plenum, New York.

Brown, M.; Harris, C. (1994): *Neuro-uzzy adaptive modeling and control*, Prentice-Hall, New Jersey.

Chang, F. J.; Wang, K. W. (2013): A systematical water allocation scheme for drought mitigation control, *Journal of Hydrology*, vol. 507, pp.124-133.

Craven, P.; Wahba, G. (1979): Smoothing noisy data with spline functions: Estimating the correct degree of smoothing by the method of generalized cross-validation. *Numerical Mathematics*, vol. 31, pp. 317-403.

DeVeaux, R. D.; Psichogios, D. C.; Ungar, L. H. (1993): A comparison of two nonparametric estimation schemes: MARS and neural networks. *Comput. Chem. Eng.*, vol. 17, no. 8, pp. 819-837.

Friedman, J. H. (1991): Multivariate Adaptive Regression Splines. Annals of Statistics, vol. 19, pp. 1-141.

Fu, A. M.; Wang, X. Z.; He, Y. L.; Wang, L. S. (2014): A study on residence error of training an extreme learning machine and its application to evolutionary algorithms. *Neurocomputing*, vol. 146, pp. 75-82.

Schutter, G. D.; Gibbs, J.; Domone, P.; Bartos, P. J. M. (2008): Self-compacting

concrete, Whittles Publishing, Dunbeath, Scotland, UK.

Gao, F.; Li, H.; Xu, B. (2013): Applications of extreme learning machine optimized by ICPSO in fault diagnosis. *Zhongguo Jixie Gongcheng/China Mechanical Engineering*, vol. 24, no. 20, pp.2753-2757.

Huang, G. B.; Babri, H. A. (1998): Upper bounds on the number of hidden neurons in feedforward networks with arbitrary bounded nonlinear activation functions. *IEEE Trans Neural Networks*, vol. 9, no. 1, pp. 224-229.

Huang, G. B.; Siew, C. K. (2004): Extreme Learning Machine: RBF Network Case. Proc. Eighth Int'l Conf. Control, Automation, Robotics, and Vision (ICARCV'04), Kunming, China, 6-9 December 2, pp. 1029-1036.

Huang, G. B.; Siew, C. K. (2005): Extreme Learning Machine with Randomly Assigned RBF Kernels. *Int'l J. Information Technology*. vol. 11, no. 1.

Huang, G. B.; Zhu, Q. Y.; Siew, C. K. (2004): Extreme Learning Machine: A New Learning Scheme of Feedforward Neural Networks. *Proc.* Int'l Joint Conf. Neural Networks, Budapest, Hungary, Jul. 25-29, 2004, 2, pp. 985-990

Huang, G. B. (2003): Learning capability and storage capacity of twohidden-layer Feedforward networks. *IEEE Trans Neural Networks*, vol. 14, no. 2, pp. 274-281.

Law, K. T.; Cao, Y. L.; He, G. N. (1990): An energy approach for assessing seismic liquefaction potential, *Canadian Geotechnical Journal*, vol. 27, no.3, pp. 320-329.

Kandel, K.; Huettmann, F.; Suwal, M. K.; Ram, R. G.; Nijman, V.; Nekaris, K. A. I.; Lama, S. T.; Thapa, A.; Sharma, H. P.; Subedi, T. R. (2015): Rapid multi-nation distribution assessment of a charismatic conservation species using open access ensemble model GIS predictions: Red panda (Ailurus fulgens) in the Hindu-Kush Himalaya region. *Biological Conservation*, vol. 181, pp.150-161.

Lee, S. H.; Lim, J. H.; Moon, K. I. (2012): An ANFIS model for environmental performance measurement of transportation. *Communications in Computer and Information Science*, vol. 352, no. 1, pp. 289-297.

Pickens, B. A.; King, S. L. (2014): Linking multi-temporal satellite imagery to coastal wetland dynamics and bird distribution. *Ecological Modelling*, vol. 285, pp. 1-12.

Pourtousi, M.; Sahu, J. N.; Ganesan, P.; Shamshirband, S.; Redzwan, G. (2015): A combination of computational fluid dynamics (CFD) and adaptive neuro-fuzzy system (ANFIS) for prediction of the bubble column hydrodynamics. *Powder Technology*, vol. 274, no. 4, pp. 466-481.

Priya, E.; Srinivasan, S.; Ramakrishnan, S. (2012): Classification of tuberculosis digital images using hybrid evolutionary extreme learning machines. *Computational Collective Intelligence*, vol. 7653, no. 1, pp. 268-277.

Rafat, S.; Paratibha, A.; Yogesh, A. (2011): Prediction of compressive strength of selfcompacting concrete containing bottom ash using artificial neural network. *Advances in Engineering Software*, vol. 42, no. 10, pp. 780-786.

Serre, D. (2002): Matrices: Theory and Applications. Springer-Verlag.

Sun, L.; Pan, Y.; Gu, W. (2013): Data mining using regularized adaptive B-splines

regression with penalization for multi-regime traffic stream models. *Journal of Advanced Transportation*, vol. 48, no. 7, pp. 876-890.

Takagi, T.; Sugeno, M. (1985): Fuzzy Identification of Systems and its Applications to Modeling, Systems, Man and Cybernetics. *IEEE Transactions*, vol. 15, no. 1, pp. 116-132.

Wang, J.; Hu, J.; Ma, K.; Zhang, Y. (2015): A self-adaptive hybrid approach for wind speed forecasting. *Renewable Energy*, vol. 78, pp. 374-385.

Yang, C. C.; Prasher, S. O.; Lacroix, R.; Kim, S. H. (2003): A Multivariate Adaptive Regression Splines Model for Simulation of Pesticide Transport in Soils. *Biosystems Engineering*, vol. 86, no. 1, pp. 9-15.

Yang, L.; Entchev, E. (2014): Performance prediction of a hybrid microgeneration system using adaptive neuro-fuzzy inference system (ANFIS) technique. *Applied Energy*, vol. 134, pp. 197-203.

Yegian, M. K.; **Marciano, E. A.** (1991): Earthquake - Induced Permanent Deformations: Probabilistic Approach. *J. Geotech. Engrg*, vol. 117, no. 1, pp. 35-50.