Nonlinear Panel Flutter Analysis Based on an Improved CFD/CSD Coupled Procedure

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Nonlinear aeroelasticity, caused by the interaction between nonlinear Abstract: fluid and geometrically nonlinear structure, is studied by an improved CFD and CSD coupled program. An AUSMpw+ flux splitting scheme, combined with an implicit time marching technology and geometric conservation law, is utilized to solve unsteady aerodynamic pressure; The finite element co-rotational theory is applied to model geometrically nonlinear two-dimensional and three-dimensional panels, and a predictor-corrector program with an approximately energy conservation is developed to obtain nonlinear structure response. The two solvers are connected by Farhat's second order loosely coupled method and the aerodynamic loads and structural displacements are transferred by boundary element method. With the application on the solution of panel flutter problems for supersonic, transonic and subsonic Mach numbers, representative limited cycle oscillations appear when geometric nonlinearity and aerodynamic nonlinearity are considered. The flutter boundary and amplitude of limit cycle oscillation are discussed and compared with the work of Dowell and Gordnier. The program is also used to compute the nonlinear elastic response of AGARD 445.6 wing, and the results show that improved procedure presents a better stability than conventional method.

Keywords: Nonlinear aeroelasticity, limited cycle oscillation, panel flutter, geometrical nonlinearity, co-rotational theory, boundary element method.

1 Introduction

Aeroelasticity is an interesting subject of the interaction between flexible structure and the surrounding fluid. Nonlinearities involved with aeroelasticity arise from two aspects: the structural and aerodynamic points of view. Such as large shock motions, flow separation and geometrical nonlinearity in structure. At present, most of the nonlinear aeroealstic analysis depends on computation of unsteady aerodynamic forces and structural linear mode superposition method, which considers

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only the nonlinearity in fluid regime. However, the structural geometrical nonlinearity plays a crucial role and the linear theory is not able to determine accurately the response in the case of large deformation for large flexible structure. There are strong requirements in discussing approaches for solving two nonlinearities involved in aeroelasticity simultaneously and studying computational interfaces adequate to handle the nonlinear interdisciplinary interactions. Studying this phenomenon, well validated computational fluid dynamics (CFD) methods should be coupled with computational structural dynamics (CSD) in an accurate way [Baum, Luol, Mestreaul, Sharov, Lohner, Prlessone and Charman (2001); Patil and Hodges (2000)], and this will provide a path for developing a comprehensive prediction capability for simulating complex, nonlinear aerodynamics and structural dynamics [Gordnier and Fithen (2003)].

Dowell [Dowell (1970); Dowell, Thomas, and Hall (2001)] classified the nonlinear aeroelasticity into four different categories and studied limit cycle oscillations (L-COs) of the panel in supersonic flow, as well as proposed reduced order aerodynamic models- proper orthogonal decomposition and the harmonic balance technique. The latter method has been developed and applied by several researchers, such as Hall[Hall, Thomas, and Clark (2002)], Dai[Dai, Schnoor, and Atluri (2012)] and Liu[Liu, Thomas, and Dowell (2007)]. Gordnier [Gordnier and Fithen (2003); Gordnier and Visbal (2004)] proposed a computational aeroelastic model consisted of a finite element solution of the nonlinear von Karman plate equations coupled to both an Euler and Navier-Stokes aerodynamic models, and performed the model on a three dimensional panel and a flexible delta wing. Mei [Mei (1977)] applied the finite element method and third-order piston theory to study the supersonic limit cycle oscillations of two-dimensional panels. Strganac [Strganac, Cizmas, and Nichkawde (2005)] studied bifurcation of vehicles with moderate-to-high deformations and presented the nonlinear structural equations of motion for cantilevered wing configurations that possess in-plane, out-of-plane, and torsional couplings, and outlined a Navier-Stokes based unsteady aerodynamic model that addressed viscous and compressible flows for transonic flows with shock/boundary layer interaction. Patil and Hodges [Patil and Hodges (2000); Patil, Hodges, and Cesnik (2001)] presented a mixed variational formulation based on the exact intrinsic equations for the dynamics of beams in moving frames and studied finite-state airloads for deformable airfoils on fixed and rotating wings, and performed the theory on High Altitude Long Endurance (HALE) aircraft aeroelastic computation. Attar [Attar and Gordnier (2005)] studied the flutter and LCO behavior of a cropped delta wing using a newly developed computational aeroelastic solver which included Euler finite difference solver and nonlinear structural model. The structural solver for geometric nonlinearities was modeled by co-rotational formulation, which was applied in aeroelastic analysis for the first time.

Most of flight vehicles' structure are generally modeled by beam or shell finite element in engineering analysis. Although many algorithms of large rotation solution have been developed as Total Lagrangian (TL) and Updated Lagrangian (UL), most of these early works were surprisingly sophisticated and worth improvement. The co-rotational approach, viewed as an effective way to describe nonlinear structural kinematics analysis, has generated an increasing amount of interest in the last decade [Crisfield (1996)]. The main idea is that the motion of the element can be split into a rigid part and a pure deformational response. Assuming the pure deformational part measured in a rotated reference frame to be small enough, a linear finite element theory can be used in the analysis [Battini (2002); Battini (2007); Pacoste (1998)]. This leads to very simple expressions for the local internal force vector and tangent stiffness matrix. Crisfield [Crisfield (1996)] developed a unified co-rotational framework for solids, shells and beams, and applied it to facet shell elements and three-dimensional beams. Battini [Battini (2002)] implemented co-rotational beam elements and branch-switching procedures in order to analyze elastic and plastic instability problems, and proposed a new numerical method based on a minimal augmentation procedure for the direct computation of elastic critical points. Battini [Battini (2007)] also did some work to modifications in triangular shell elements. Pacoste [Pacoste (1998)] investigated the formulation of co-rotational flat facet triangular elements for the numerical analysis of instability phenomena in shell structures considered three types of local formulations. Cai [Cai, Paik and Atluri (2009)] and Zhu [Zhu, Cai, Paik and Atluri (2010)] proposed an updated Lagrangian corotational frame for analyzing large rotations, and large deformation of plates and shells.

For structural dynamic response solution, Galvaneito and Crisfield [Galvaneito and Crisfield (1996)] presented an exact energy-conserving procedure for the implicit nonlinear dynamic analysis of planar beam structures. Chimakurth [Chimakurth and Cesnik (2011)] utilized a nonlinear finite element solver based on a co-rotational approach and simulated flapping wing structure under large displacements and rotations. Relvas [Relvas and Suleman (2007); Relvas and Suleman (2006)] applied the finite element co-rotational theory to model geometrically nonlinear structures and utilized a dynamic co-rotational energy-conserving algorithm to solve the nonlinear structural response. The unsteady Euler solver and vortexring method were used to model aerodynamic loads for a two-dimensional airfoil and a plate under low speed airflow respectively.

In subsystem, the CFD grid is different from the CSD grid on account of the different reference frame between Euler and Lagrange. In aeroelastic analysis, there are two kinds of data transfer on common boundary: one is interpolating the displacements calculated by the CSD solver to CFD grid, and the other is transforming the aerodynamic loads computed by CFD code on CSD points. At present, the most popular interpolation methods [Goura and Badcock (2001); Kim and Kim (2005); Sadeghi, Liu, and Lai (2004)] can be categorized as two groups: (1) Surface fit-ting method, which utilizes a global function with weighting coefficients, such as Infinite-Plate Spline (IPS), this kind of method has dominantly used for displacement transfer; (2) Surface tracking method, which utilizes the shape functions of finite elements by projecting fluid nodes to nearest structural elements, for example Conservation Volume Transformation (CVT). Currently, surface tracking method is mostly used for load transfer.

However, the above popular methods: IPS and CVT cannot keep energy conservation on the boundary between CFD and CSD module. The boundary element method (BEM), which was generally used for solid mechanics analysis, was firstly proposed by Chen and Jadic [Chen and Jadic (1998)] to generate the universal spline matrix for CFD/CSD data transfer. Then, BEM scheme was developed to deal with complex configuration, including structural discontinuity like control surfaces by Chen and Gao [Chen and Gao (2001)]. The approach was developed to handle large and complex structures and transform the displacement vectors at a solid boundary to the interior of the field grid as mesh deformation algorithm by Lai [Lai, Tsai, and Lum (2002); Lai, Tsai, and Liu (2003)]. It is a pity that the BEM technique has not been employed in aeroelastic simulation in any literatures.

So far, time marching algorithm of CFD/CSD coupled method can be classified broadly under four major categories [Kamakoti and Shyy (2004); Farhat and Van (2006)](as seen in Fig. 1):

- (i) Fully coupled method can solve the two solvers simultaneously and accurately, however, it is highly nonlinear and limited to two-dimensional problems.
- (ii) Loosely coupled method has advantages of simplify explicit/implicit treatment, sub-iteration and software modularity, but it is only first order time accuracy, therefore, the time-step requires small enough.
- (iii) Tightly coupled method is developed to eliminate the time-lag, and it can achieve second order time accuracy, however, the computational cost increase greatly.
- (iv) The fourth CFD/CSD coupled procedure in Fig. 1, named as second order loosely coupled method, was proposed by Farhat [Farhat and Van (2006);Geuzaine, Van, and Farhat (2004)] and proved to reach second order time accuracy if the solver of the subsystem was selected appropriately. It has the same advantages of the loosely coupled method.



Figure 1: Description of existing CFD/CSD coupled procedures. (a)fully coupled method, (b) loosely coupled method, (c) tightly coupled method, (d) second order loosely coupled method

In the present article, the CSD solver will use a predictor-corrector program with an approximate energy conservation algorithm to solve the nonlinear structure response. The CFD solver will utilize an AUSMpw+ flux splitting scheme, an implicit dual-time stepping technology and the geometric conservation law to solve Euler and Reynolds averaged Navier-Stokes equation. We use the BEM method which is most robust and accurate for energy mapping conservation by combining of the load and displacement transfer, and we will apply Farhat's second order loosely coupled procedure for connecting the CFD program with CSD solver. This improved program is used to analyze the flutter characteristics of two-dimensional and three-dimensional panels for supersonic, transonic and subsonic Mach numbers, as well performed on the solution of nonlinear elastic response of AGARD 445.6 wing.

2 Unsteady Aerodynamic Load Model

Euler equation and Reynolds averaged Navier-Stokes equation are applied to establish the model for solving aerodynamics, taking Euler equation as an example:

$$\frac{\partial}{\partial x} \int \bar{\mathbf{U}} d\Omega + \int \bar{\mathbf{F}} \cdot d\mathbf{S} = 0 \tag{1}$$

Where $\bar{\mathbf{U}}$ are the fluid variables, $\bar{\mathbf{F}}$ denote the vector of inviscid convective flux. $d\Omega$ and $d\mathbf{S}$ are the moving control finite volume and outer normal area vector of the control surface respectively. An AUSMpw+ flux splitting scheme [Yao and Xu (2008)] is utilized to discretize the inviscid flux as follows:

$$\bar{\mathbf{F}}_{1/2} = \bar{M}_L^+ c_{1/2} \bar{\mathbf{U}}_L + \bar{M}_R^- c_{1/2} \bar{\mathbf{U}}_R + \left(\Psi_L^+ p_L + \Psi_R^- p_R\right)$$
(2)

Where *c* is sound velocity, *p* is flow pressure and $\Psi^{\pm} = \Psi^{\pm}(M)$, where *M* is Mach number.

Define $m_{1/2} = c_{1/2}M_{1/2}$, \bar{M}_L^+ and \bar{M}_R^- can be written as: When $0 < m_{1/2} \le 1$

$$\begin{cases} \bar{M}_{L}^{+} = M_{L}^{+} + M_{R}^{-} \left((1 - \omega) \left(1 + f_{R} \right) - f_{L} \right) \\ \bar{M}_{R}^{-} = M_{R}^{-} \omega \left(1 + f_{R} \right) \end{cases}$$
(3)

When $-1 \le m_{1/2} < 0$

$$\begin{cases} \bar{M}_{L}^{+} = M_{L}^{+} \omega (1 + f_{L}) \\ \bar{M}_{R}^{-} = M_{R}^{-} + M_{L}^{+} \left((1 - \omega) (1 + f_{L}) - f_{R} \right) \end{cases}$$
(4)

Where ω is defined as

$$\boldsymbol{\omega} = 1 - \min\left(\frac{p_L}{p_R}, \frac{p_R}{p_L}\right) \tag{5}$$

The modified pressure weighted function f is defined as follows:

$$f_{L,R} = \begin{cases} \frac{p_{L,R}}{\Psi_L^+ p_L + \Psi_R^- p_R} - 1 & |M_{L,R}| < 1\\ 0 & |M_{L,R}| \ge 1 \end{cases}$$
(6)

In order to solve unsteady flow, a sub-iterative course of the dual-time technology is introduced to eliminate the time marching error. Suppose that fluid grid moves with the body flexibly, and then the introduced dual-time term can be written as:

$$\Omega \frac{d\bar{\mathbf{U}}}{d\tau} + \frac{3\Omega^{n+1}\bar{\mathbf{U}}^{n+1} - 4\Omega^n\bar{\mathbf{U}}^n + \Omega^{n-1}\bar{\mathbf{U}}^{n-1}}{2dt} + \bar{\mathbf{Q}}^{n+1} = 0$$
(7)

Where $\bar{\mathbf{Q}}$ is computed by $\sum \bar{\mathbf{F}} \cdot d\mathbf{S}$, τ and *t* are pseudo and real time respectively. Transfinite interpolation (TFI) technique [Wong and Tsai (2000)] is applied to treat the moving grid of unsteady flow, and the geometric conservation law (GCL) is introduced to solve the moving finite volume as follows:

$$\Omega^{n+1} = \frac{4}{3}\Omega^n - \frac{1}{3}\Omega^{n-1} + \frac{2\Delta t}{3}\int \mathbf{V} \cdot d\mathbf{S}$$
(8)

Where **V** is the velocity of the moving grid.

Note that the above program also can be used to solve Reynolds averaged Navier-Stokes equation.



Figure 2: Panel geometry

3 Geometrically Nonlinear Model of Panel

A flexible panel of length *a*, width *b*, thickness *h*, and mass density ρ_s shown in Fig. 2 is investigated in the present work. The air flowing above the panel at Mach number M_{∞} and density ρ_{∞} is in the positive *x* direction. Two cases are studied: two-dimensional case as a/b = 0 and three-dimensional case as a/b = 1. The panel's characteristics are: h/a = 0.002, mass ratio $\mu_s = \rho_{\infty} a/\rho_s h = 0.1$, Poisson's ration v = 0.3 and modulus of elasticity $E_s = 7.0 \times 10^{10}$. The panel is pinned at both edges in the flow direction.

3.1 Two-Dimensional Panel Model

For two-dimensional case a/b = 0, the panel can be considered as planar beam structure, which can be described by co-rotational frame as shown in Fig. 3 [Cr-isfield (1996); Battini (2002)].

The vectors of global and local displacements of the element are defined by d and d_l as

$$\begin{cases} d = \begin{bmatrix} u_1 & w_1 & \theta_1 & u_2 & w_2 & \theta_2 \end{bmatrix}^T \\ d_l = \begin{bmatrix} \bar{u} & \bar{\theta}_1 & \bar{\theta}_2 \end{bmatrix}^T \end{cases}$$
(9)

Where $\bar{u} = l_c - l_0$.

The vectors of global and local inter forces of the element are defined by F_i and f_l as

$$\begin{cases} F_{i} = \begin{bmatrix} X_{1} & Z_{1} & M_{1} & X_{2} & Z_{2} & M_{2} \end{bmatrix}^{T} \\ f_{l} = \begin{bmatrix} \bar{N} & \bar{M}_{1} & \bar{M}_{2} \end{bmatrix}^{T} \end{cases}$$
(10)

Let $c = cos\beta$ and $s = sin\beta$, through differentiation of Eq. 9, and utilize geometry connection, then transformation matrix **T** between local and global displacements



Figure 3: Co-rotational frame of two-dimensional panel

can be obtained by

$$\delta d_l = \mathbf{T} \delta d \tag{11}$$

Where

$$\mathbf{T} = \begin{bmatrix} -c & -s & 0 & c & s & 0 \\ -s/l_c & c/l_c & 1 & s/l_c & -c/l_c & 0 \\ -s/l_c & c/l_c & 0 & s/l_c & -c/l_c & 1 \end{bmatrix}$$

The relation between global and local inter forces can be obtained by virtual work

$$F_i = \mathbf{T}^T f_l \tag{12}$$

Through differentiation of Eq. 12, we can obtain

$$\delta F_i = \mathbf{T}^T \delta f_l + f_l^T \delta \mathbf{T} \tag{13}$$

The first term in Eq. 13 is computed by introducing the local standard linear stiffness matrix \mathbf{K}_l

$$\delta f_l = \mathbf{K}_l \delta d_l = \mathbf{K}_l \mathbf{T} \delta d \tag{14}$$

Introduce two notations

$$\begin{cases} \mathbf{r} = \begin{bmatrix} -c & -s & 0 & c & s & 0 \end{bmatrix}^T \\ \mathbf{p} = \begin{bmatrix} s & -c & 0 & -s & c & 0 \end{bmatrix}^T \end{cases}$$
(15)



Figure 4: Co-rotational frame of three-dimensional panel

Then utilize their differentiations, the second term in equation 13 can be obtained and the tangent stiffness equation can be finally written as

$$\mathbf{K}_{t} = \mathbf{T}^{T} \mathbf{K}_{l} \mathbf{T} + \mathbf{K}_{\sigma m}$$
(16)

Where $\mathbf{K}_{\sigma m}$ is called geometric matrix and expressed as

$$\mathbf{K}_{\sigma m} = \frac{\bar{N}}{l_c} \mathbf{p} \mathbf{p}^T + \frac{\bar{M}_1 + \bar{M}_2}{l_c^2} \left(\mathbf{r} \mathbf{p}^T + \mathbf{p} \mathbf{r}^T \right)$$
(17)

3.2 Three-Dimensional Panel Model

For three-dimensional case a/b = 1, the panel can be modeled by triangular shell structure as described in Fig. 4.

The origin is taken at the geometric center point C, and an orthogonal matrix to specify the orientation of the local frame is defined as

$$\mathbf{R}_0 = \begin{bmatrix} e_1 & e_2 & e_3 \end{bmatrix} \tag{18}$$

Where e_i denote the three axes vectors. The rigid translation and rotation are described by the displacements u_c^g of node *C* and orthogonal matrix \mathbf{R}_r respectively. The nodal local pure deformational values are defined by \bar{u}_i and $\bar{\mathbf{R}}_i$ as shown in Fig. 4.

Then, the local displacements can be derived as

$$\begin{cases} \bar{u}_i = \mathbf{R}_r^T \left(r_i^g + u_i^g - r_c^g - u_i^g \right) - r_i^0 \\ \bar{\mathbf{R}}_i = \mathbf{R}^T \mathbf{R}_i^g \mathbf{R}_0 \end{cases} \quad i = 1, 2, 3$$
(19)

Where u_i^g are nodal displacements in global frame, r_c^g are the original coordinates of node *C*, and r_i^0 are the vectors from three corners to node *C*. Pacoste [Pacoste (1998)] proposed a parameterization of the orthogonal matrix to represent large rotation, the global rotation matrix of each point can be defined as

$$\mathbf{R}_{i}^{g} = \exp\left(\tilde{\psi}_{i}\right) \tag{20}$$

The rotational values of angular variable and rotational vector can be related as

$$\delta \theta_i^g = \mathbf{T}_m(\boldsymbol{\psi}_i) \, \delta \boldsymbol{\psi}_i \tag{21}$$

Where

$$\mathbf{T}_{m}(\boldsymbol{\psi}_{i}) = \frac{\mathbf{R}_{i}^{g} + \mathbf{I}}{\sqrt{1 - (q_{1}^{2} + q_{2}^{2} + q_{3}^{2})}}$$

The local displacements and internal forces can be written as

$$\begin{cases} d_l = \begin{bmatrix} \bar{u}_1^T & \bar{\theta}_1^T & \bar{u}_2^T & \bar{\theta}_2^T & \bar{u}_3^T & \bar{\theta}_3^T \end{bmatrix}^T \\ f_l = \begin{bmatrix} \bar{F}_1^T & \bar{M}_1^T & \bar{F}_2^T & \bar{M}_2^T & \bar{F}_3^T & \bar{M}_3^T \end{bmatrix}^T \end{cases}$$
(22)

In local coordinate system, the above internal forces and displacements can be expressed in linear relation as

$$f_l = \mathbf{K}_l d_l \tag{23}$$

Where \mathbf{K}_l is the linear triangular shell element local stiffness matrix. The global displacements and forces are defined as

$$\begin{cases} d_g = \begin{bmatrix} u_1^T & \theta_1^T & u_2^T & \theta_2^T & u_3^T & \theta_3^T \end{bmatrix}^T \\ F_g = \begin{bmatrix} F_{g1}^T & M_{g1}^T & F_{g2}^T & M_{g2}^T & F_{g3}^T & M_{g3}^T \end{bmatrix}^T \end{cases}$$
(24)

Through differentiation of Eq. 23, and utilize geometry connection, then transformation matrix between local and global frame can be obtained by

$$\begin{cases} \delta d_l = \mathbf{T}_g \delta d_g \\ F_g = \mathbf{T}_g^T f_l \end{cases}$$
(25)

Where

 $\mathbf{T}_g = \mathbf{P}\mathbf{E}^T$

$$\mathbf{P} = \begin{bmatrix} \frac{\partial \bar{u}_i}{\partial u_i^e} & \frac{\partial \bar{u}_i}{\partial \theta_i^e} \\ \frac{\partial \theta_r^e}{\partial u_i^e} & \frac{\partial \theta_r^e}{\partial \theta_i^e} \end{bmatrix}$$
$$\mathbf{E} = diag (\mathbf{R}_r, \mathbf{R}_r, \mathbf{R}_r, \mathbf{R}_r, \mathbf{R}_r, \mathbf{R}_r)$$

The above variables are computed by the defined spatial angular variation, it has to transform the rotational values to rotational vector. New definitions of the displacements and internal forces are given as

$$\begin{cases} d = \begin{bmatrix} u_1^T & \psi_1^T & u_2^T & \psi_2^T & u_3^T & \psi_3^T \end{bmatrix}^T \\ F = \begin{bmatrix} F_1^T & M_1^T & F_2^T & M_2^T & F_3^T & M_3^T \end{bmatrix}^T$$
(26)

Via virtual work, the new internal forces can be obtained by

$$F = \mathbf{B}_m^T F_g = \mathbf{T}^T f_l \tag{27}$$

Where

$$\mathbf{T} = \mathbf{T}_{g} \mathbf{B}_{m}$$
$$\mathbf{B}_{m} = diag \begin{pmatrix} \mathbf{I}_{3} & \mathbf{T}_{m}(\psi_{1}) & \mathbf{I}_{3} & \mathbf{T}_{m}(\psi_{2}) & \mathbf{I}_{3} & \mathbf{T}_{m}(\psi_{3}) \end{pmatrix}$$

Through differentiation of Eq. 27, the tangent stiffness matrix can be finally written as

$$\mathbf{K}_{t} = \mathbf{T}^{T} \mathbf{K}_{l} \mathbf{T} + \mathbf{K}_{\sigma m}$$
⁽²⁸⁾

Where

$$\mathbf{K}_{\sigma m} = diag \begin{pmatrix} 0_3, & \mathbf{K}_{\sigma m 1}, & 0_3, & \mathbf{K}_{\sigma m 2}, & 0_3, & \mathbf{K}_{\sigma m 3} \end{pmatrix}$$
$$\mathbf{K}_{\sigma m i} = \frac{\partial \mathbf{T}_m^T(M_{gi})}{\partial \psi_i} \quad i = 1, 2, 3$$

4 An Approximate Energy Conservation Algorithm

Newmark integral approach, an unconditional stable and predominant procedure to determine the structure dynamic response at each time step for linear problems, may encounter "lock" or "blow up" solution for nonlinear case. It has been verified by Galvanetto and Crisfield [Galvaneito and Crisfield (1996)], and an energy conservation algorithm was proposed for the implicit nonlinear dynamic analysis of planar beam structures in literature [Galvaneito and Crisfield (1996)], in which the time step $n + \frac{1}{2}$ was used to achieve energy conservation during *n* to n + 1 step. However, it may appear more complex and more data have to be stored. Based on this algorithm, an approximate energy conservation algorithm is developed, which retains the stability and accuracy, and simplifies the procedure without any modification.

Assume that there is no damp in the structural system, the equilibrium equation at n+1 step can be written as

$$g_{n+1} = F_{i,n+1} + \mathbf{M}\ddot{u}_{n+1} - F_{s,n+1} = 0$$
⁽²⁹⁾

Where **M** and $F_{s,n+1}$ are the mass matrix and the external force as unsteady aerodynamic load respectively. By introducing midpoint rule and the variables of $n + \frac{1}{2}$ step as

$$F_{i,n+1/2} = \mathbf{T}_{n+1/2}^T f_{il,n+1/2} \tag{30}$$

Where

$$\mathbf{T}_{n+1/2} = \frac{1}{2} \left(\mathbf{T}_n + \mathbf{T}_{n+1} \right)$$

The equilibrium equation at $n + \frac{1}{2}$ step can be derived as

$$g_{n+1/2} = F_{i,n+1/2} + \frac{1}{\Delta t} \mathbf{M} \left(\dot{u}_{n+1} - \dot{u}_n \right) - F_{s,n+1/2} = 0$$
(31)

Let $g_{n+1/2} \rightarrow 0$, then the energy change between *n* and n+1 step $\Delta E = g_{n+1/2}\Delta u_{n+1/2}$ could be conservative. Differentiate the Eq. 30 and the new global tangent stiffness matrix can be obtained as

$$\mathbf{K}_{t,n+1/2} = \frac{1}{2} \left(\frac{\mathbf{T}_n + \mathbf{T}_{n+1}}{2} \right)^T \mathbf{K}_l \mathbf{T}_{n+1} + \frac{1}{2} \mathbf{K}_{\sigma m}$$
(32)

It is notable that Eq. 32 can be used for both beam and shell structure.

4.1 Predictor Step

The initial values of displacement increment are determined by

$$\mathbf{\bar{K}}_{t,n+1/2}\Delta u = \Delta F \tag{33}$$

Where

$$\bar{\mathbf{K}}_{t,n+1/2} = \mathbf{K}_{t,n+1/2} + \frac{2}{\Delta t^2} \mathbf{M},$$
$$\Delta F = F_{s,n+1/2} - F_{i,n+1/2} + \frac{2}{\Delta t} \mathbf{M} \dot{u}_n$$

Then the information at n + 1 step can be predicted by

$$\begin{cases} u_{n+1} = u_n + \Delta u \\ \dot{u}_{n+1} = \frac{2}{\Delta t} \Delta u - \dot{u}_n \end{cases}$$
(34)

4.2 Corrector Step

Firstly, $g_{n+1/2}^{i-1}$ can be received from Eq. 31, and a new equilibrium equation is applied as

$$g_{n+1/2}^{i-1} + \bar{\mathbf{K}}_{t,n+1/2}^{i-1} \delta u_{n+1}^{i} = 0$$
(35)

Secondly, the displacements and velocities can be updated by δu_{n+1}^i as

$$\begin{cases} u_{n+1}^{i} = u_{n+1}^{i-1} + \delta u_{n+1}^{i} \\ \dot{u}_{n+1}^{i} = \dot{u}_{n+1}^{i-1} + \delta \dot{u}_{n+1}^{i} \end{cases}$$
(36)

Where $\delta \dot{u}_{n+1}^i = \frac{2}{\Delta t} \delta u_{n+1}^i$.

The above two equations 35 and 36 should be solved repeatedly in the corrector process until a convergence criterion with $g_{n+1/2} \rightarrow 0$ is satisfied.

5 Data Transfer Via Boundary Element Method

If the displacements x on the entire boundary (CFD surface grid) are known, a transformation from boundary to interior displacements u of the internal source (CSD grid) can then be performed by a transformation matrix **B** as

$$u = \mathbf{B}x, \mathbf{B} = \mathbf{G}_{as}\mathbf{G}_{aa}^{-1}\mathbf{H}_{aa} - \mathbf{H}_{as}$$
(37)

Where **H** and **G** are formed from the kernel integral of the displacement and traction, and the subscript *a* and *s* stand for CFD surface grid and CSD grid values respectively. Via the minimum strain energy ($W = x_a^T \mathbf{R} x_a$) requirement [Chen and Jadic (1998), the spline matrix is determined by

$$\mathbf{D} = \left(\mathbf{R} + \mathbf{R}^{T}\right)^{-1} \mathbf{B}^{T} \left[\mathbf{B} \left(\mathbf{R} + \mathbf{R}^{T}\right)^{-1} \mathbf{B}^{T}\right]^{-1}$$
(38)

By embracing all CFD and CSD gird points, an intermediate BEM model is constructed as a third platform [Chen and Gao (2001)]. Firstly, a universal spline matrix **D** can be generated to transfer the information of the CSD grid to the intermediate BEM model by Eq. 38, then a BEM matrix **B** is constructed to transfer the information on the intermediate BEM model to the CFD grid by Eq. 37. Finally the mapping matrix between two different grid systems can be obtained

$$\mathbf{S} = \mathbf{B}\mathbf{D} \tag{39}$$

Thus, the structural displacements and aerodynamic loads can be transferred by

$$\begin{cases} x = \mathbf{S}u\\ F_s = \mathbf{S}^T F_a \end{cases}$$
(40)

Where u and x are the displacements of structural and CFD surface grid respectively. F_s and F_a denote the vector of structural nodal forces and aerodynamic loads respectively. Thus, the requirements of the conservation of work between two transformations can be satisfied as

$$W_s = F_s^T u_s = \left(\mathbf{S}^T F_a\right)^T u_s = F_a^T \left(\mathbf{S} u_s\right) = F_a^T u_a = W_a \tag{41}$$

6 CFD/CSD Coupled Program

Based on the Farhat's second order loosely coupled procedure [Farhat and Van (2006)], the two solvers are connected as following six steps:

(i) Predict the structural displacements at time-step $n + \frac{1}{2}$ by *n* step

$$u_{n+1/2} = u_n + \frac{\Delta t}{4} \left(\dot{u}_n + \frac{\Delta u_n}{\Delta t} \right) \tag{42}$$

- (ii) Transfer the predicted motion $u_{n+1/2}$ to the fluid system as $x_{n+1/2}$ by Eq. 40.
- (iii) Update the position of the fluid grid by TFI technique and compute the new control finite volume by Eq. 8.
- (iv) Solve Eq. 7 to obtain the loads on aerodynamic surface.
- (v) Convert the aerodynamic loads $F_{a,n+1/2}$ into structure element to get equivalent loads $F_{s,n+1/2}$ by Eq. 40.
- (vi) Advance Eq. 31 and approach the motion at n + 1 step by the predictorcorrector program.

7 Numerical Results

7.1 CFD Solver for AGARD CT5 Unsteady Test

Two cases of AGARD CT5 unsteady test are introduced to validate the present CFD solver. Unsteady motion of the NACA0012 airfoil is defined as

$$\alpha(t) = \alpha_m + \alpha_0 \sin(2kt) \tag{43}$$



Figure 5: (a)Lift coefficient vs. angle of attack in case 1, (b) moment coefficient vs. angle of attack in case 1



Figure 6: (a)Lift coefficient vs. angle of attack in case 2, (b) moment coefficient vs. angle of attack in case 2

Flow conditions are set as follows: (1) Case1: $M_{\infty} = 0.6$, $\alpha_m = 3.16^{\circ}$, $\alpha_0 = 4.59^{\circ}$ and k = 0.0811. The axis position is 0.273 chord. (2) Case2: $M_{\infty} = 0.755$, $\alpha_m = 0.016^{\circ}$, $\alpha_0 = 2.51^{\circ}$ and k = 0.0814. The axis position is 0.25 chord.

In both two cases, the CFD gird is constructed by O grid consisting of 121×80 points and Euler equation is solved with the total time step numbers in one period is 80, and the number of sub-iteration within one time step is 10. The computed results of the lift and moment coefficient in Fig. 5 and 6 are the first 7 periods. The results are also compared with experimental values. The computation demonstrates reasonable agreement with the experimental data for both two cases.



Figure 7: (a)Cantilever beam subjected to an end moment, (b) cantilever beam nonlinear static analysis results

7.2 Cantilever Beam Nonlinear Static Analysis

The cantilever beam subjected to an end moment (shown in Fig. 7(a)) which has an analytical solution is solved by present finite co-rotational formulation.

The beam's parameters are defined as: l = 20m, $EI_z = 10N \cdot m$ and EA = 10N. The end moment *M* is set to 0.2π , 0.4π , 0.6π , 0.8π and π respectively. While the exactly analytical solution is expressed as $\theta = (MI) / (EI_z)$. The results presented in Fig. 7(b) and Tab. 1 agree well with the analytical solution.

Table 1: Comparison of the tip displacements of cantilever beam between corotational formulation and exactly analytical solution

M	Co-rotational	Exactly	Co-rotational	Exactly
	formulation	analytical	formulation	analytical
	(u/l)	solution (u/l)	(w/l)	solution (w/l)
0.2π	-0.243	-0.243	-0.550	-0.550
0.4π	0.768	-0.766	-0.720	-0.720
0.6π	-1.158	-1.156	-0.477	-0.478
0.8π	-1.189	-1.189	-0.136	-0.137
π	-1.0	-1.0	0.0	0.0

7.3 Hinged cylindrical Segment Nonlinear Dynamic Simulation

The nonlinear problems of a hinged cylindrical segment with a total thickness t = 0.00635m presented in Fig. 8 are analyzed. The total length and circumferential



Figure 8: Geometry of a hinged cylindrical segment

length are $L_a = L_b = 0.508m$, and curvature of the circular edges is R = 2.54m. The material properties are $E = 3.105 \times 10^9 N \cdot m$ and $\mu = 0.3$. The hinged cylindrical segment is taken under the constant central point load P = 500N. The time step is set to $\Delta t = 0.0001s$ and $\Delta t = 0.001s$ respectively. Then the dynamic responses at point A are computed by the linear Newmark method, nonlinear Newmark method and the improved predictor-corrector procedure with the approximate energy conservation algorithm.

The time histories of the normal displacement and total energy change via various schemes are shown in Fig. 9. It can be seen that the linear results are obviously different from the other two nonlinear results as the nonlinearity has much affection in the structure. The nonlinear Newmark method has the similar response to the improved scheme in original time steps. However, with time boosting, nonlinear Newmark method tends to augment after some time steps. Also, the nonlinear Newmark scheme has wide variations in total energy changes while the improved procedure tends to zero, which keeps the energy conservation. It is evident that the conventional nonlinear Newmark integration has a limited stability in the nonlinear case.

When the time step is made larger with $\Delta t = 0.001s$, as shown in Fig. 10, the linear results also present error curve, and the two nonlinear results show obvious difference. The amplitude of the nonlinear Newmark method is increased markedly and ends up with high energy state, about $8 \times 10^4 J$, which leads to a lack of convergence and provides improper information for designers. Clearly, improved predictor-corrector procedure is keeping stable versus time steps, and can be set to larger time steps and gives satisfactory results.

7.4 Nonlinear Panel Flutter Analysis

(1)Two-dimensional panel flutter results

The finite element model composes of 21 Bernoulli beam elements. The CFD



Figure 9: (a)Time-histories of normal displacement at $\Delta t = 0.0001s$, (b) timehistories of total energy change at $\Delta t = 0.0001s$



Figure 10: (a)Time-histories of normal displacement at $\Delta t = 0.001s$, (b) timehistories of total energy change at $\Delta t = 0.001s$

grid is constructed by H grid consisting of 161×41 points, and Euler equation is solved with the number of sub-iteration within one time step is 10. Mach numbers are chosen as: supersonic case $M_{\infty} = 1.2$, $M_{\infty} = 1.414$, $M_{\infty} = 1.8$ and $M_{\infty} = 2.0$; transonic and subsonic case $M_{\infty} = 1.02$, $M_{\infty} = 0.95$, $M_{\infty} = 0.9$ and $M_{\infty} = 0.7$. The flutter computation are initiated as $\dot{w} = \dot{w}_0 sin(\pi x)$.

Fig. 11 and Fig. 12 show the time-histories of the oscillation and phase portrait of w/h vs. \dot{w}/h under dynamic pressure $\lambda = \rho_{\infty} V_{\infty}^2 a^3/D = 100$ for $M_{\infty} = 1.2$ case at x/a = 0.75. It is clear that a representative limited cycle oscillation appears when the nonlinearity is considered. Fig. 13 shows the limit cycle oscillation amplitude values of Dowell(linearized potential theory), Gordnier(Euler equation) and present work for a supersonic Mach number at x/a = 0.75. When $M_{\infty} = 1.2$, the three amplitude values are compared well with each other though Dowell used linearized potential theory. For $M_{\infty} = 1.414$ case, the results have good agreement with Gordnier's work; these two solutions have lower values than Dowell's results. For a higher Mach number $M_{\infty} = 1.8$, there are small difference between the present work and Gordnier's solution. When $M_{\infty} < 1.0$, static equilibrium positions are obtained under a broad range of dynamic pressure λ . The results are compared well with Gordnier' work at x/a = 0.5 as shown in Fig. 14. In the present work, only one static equilibrium values are obtained for a same initial condition $\dot{w} = \dot{w}_0 \sin(\pi x)$. Fig. 15 shows the flutter boundary. Note that it will be singular when $M_{\infty} = 1.0$ in Dowell' solution.

(2) Three-dimensional panel flutter results

A finite element structural model is constructed 400 triangular shell elements and 1323 degrees of freedom. The CFD grid is constructed by H grid consisting of $121 \times 121 \times 31$. In this case, Euler equation is still solved with the number of subiteration within one time step is still 10. Mach numbers are chosen as: supersonic case $M_{\infty} = 1.2$, $M_{\infty} = 1.414$ and $M_{\infty} = 1.6$. Fig. 16 shows that the time-histories of the oscillation under $\lambda = 300$ for case at x/a = 0.75. Note that the peak values have a small variety in every cycle. Fig. 17 shows the pressure on section under four different positions. The curves for the two peak position are nearly symmetric. Fig. 18 shows the limit cycle oscillation amplitude values of Dowell, Gordnier and present work for three-dimensional panel at x/a = 0.75. For $M_{\infty} = 1.2$ case, the three amplitude values are also compared well with each other. While for higher Mach numbers: $M_{\infty} = 1.414$ and $M_{\infty} = 1.6$, there are evident difference between the present work and Dowell's solution as we apply nonlinear aerodynamic solver.

7.5 Nonlinear Aeroelastic Response of AGARD 445.6 Wing

The simulation of the nonlinear aeroelastic response of the AGARD 445.6 wing (constructed by NACA 65A004 airfoil) is also considered to test the prediction



Figure 11: Time-histories of the oscillation at x/a = 0.75



Figure 12: Phase portrait of w/h vs. \dot{w}/h at x/a = 0.75



Figure 13: Limit cycle oscillation amplitude vs. dynamic pressure for 2D panel at x/a = 0.75



Figure 14: Static deflection vs. dynamic pressure at x/a = 0.5



Figure 15: Dynamic pressure vs. Mach number



Figure 16: Time-histories of the oscillation at x/a = 0.75



Figure 17: Pressure of panel under four position at x/a = 0.75



Figure 18: Limit cycle oscillation amplitude vs. dynamic pressure for 3D panel at x/a = 0.75



Figure 19: (a)Sketch of AGARD 445.6 wing configuration, (b) CFD model with intermediate BEM model

capability for more complex configuration of the developed program. The material properties of the flexible wing are $E_1 = 3.15Gpa$, $E_2 = 0.42Gpa$, $\mu = 0.3$, G = 0.44Gpa and $\rho = 382kg/m^3$. Fig. 19(a) shows the configuration of the wing. Fig. 19(b) shows the CFD surface model surrounded by intermediate BEM model. A finite element structural model is constructed 200 triangular composite shell elements and 363 degrees of freedom. The CFD grid is constructed by C grid consisting of $159 \times 45 \times 45$, the surface mesh contains 2940 points. In this case, Reynolds averaged Navier-Stokes equation is considered to instead of Euler equation, and B-L turbulence model is used. The number of sub-iteration within one time step is 10.

The free stream condition are $M_{\infty} = 1.05$, $\alpha = 5^{\circ}$ and $q_{\infty} = 8171 kg/(m \cdot s^2)$, and the time step is set to $\Delta t = 0.0002s$. The aeroelastic responses of the wing under large aerodynamic loads are computed by the coupled system. In structural solution, the developed predictor-corrector procedure with approximate energy conservation algorithm, linear Newmark method and linear mode superposition with first four modes are considered. Fig. 20 shows the displacement curves of normal direction and phase portrait normal displacement vs. velocity at the point on trailing edge tip by three structural dynamic algorithms. It can be seen that both two linear algorithms present the similar augment amplitude whereas the developed nonlinear approach tends to the same variation after some time steps. A representative LCO appears when geometrically nonlinear effect is considered. It means that the nonlinearity can alleviate the divergence of the structural response.

Fig. 21 shows the histories of lift and drag coefficient of the wing. It can be seen that the lift coefficient curve of the improved predictor-corrector nonlinear algorithm has a similar LCO phenomenon to the above structural response in Fig. 20, while



Figure 20: (a)Time-histories of normal displacement on trailing edge tip, (b)phase portrait normal displacement vs. velocity on trailing edge tip

the linear results tend to diverge. From Fig. 21, the drag coefficient of the present method shows a cycle oscillation characteristic, however, the two linear methods show markedly different trends with time marching. It indicates that the error caused by a linear approach in structural solver will induce a much discrepancy in fluid solution, which has a sensitive effect for fluid convergence.



Figure 21: (a)Time-histories of lift coefficient of the wing, (b)time-histories of drag coefficient of the wing

Fig. 22 shows normal displacements and loads distribution of the wing at time t = 0.11875s via three methods. It is obvious that the wing has small displacements in X and Y direction on the tip when considering geometrical nonlinearity. When

considering the deformation and loads distribution in aeroelastic analysis, the linear mode superposition method may be of no avail, and the linear Newmark will be useless for a nonlinear solution under large load and deformation.



Figure 22: (a)Normal displacements distribution of the wing at t = 0.11875s, (b)Normal loads distribution of the wing at t = 0.11875s

8 Conclusions

In this paper, we design an improved CFD/CSD coupled procedure for solving both aerodynamic and structural nonlinearity involving four parts:

- (i) CFD solver-AUSMpw+ flux splitting scheme with dual-time implicit technology and geometric conservation law are introduced. It can solve Euler and Reynolds averaged Navier-Stokes equation and obtain unsteady aerodynamic load.
- (ii) CSD solver-a predictor-corrector program with an approximate energy conservation algorithm based on co-rotational theory is developed. It can treat with geometrically nonlinear dynamic response of the beam and shell structure.
- (iii) Data transfer between two different solvers is performed by the BEM method, which keeps energy conservation accurately and combines of the load and displacement transformation together.
- (iv) A second order loosely coupled method is applied to manage time marching algorithm between CFD and CSD solver.

It is evident that the predictor-corrector procedure with an approximate energy conservation algorithm based on co-rotational frame in our CSD solver can deal with geometrically nonlinear dynamic structure effectively. Combined with finite volume method and dual-time technology, and through CFD/CSD coupled algorithm, the improved procedure can capture the representative LCO behavior for the solution of two-dimensional, three-dimensional panel and a complex wing, and it will provide a path for developing a comprehensive prediction capability for simulating complex, nonlinear aerodynamics and structural dynamics.

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